

Solution of HW # 15

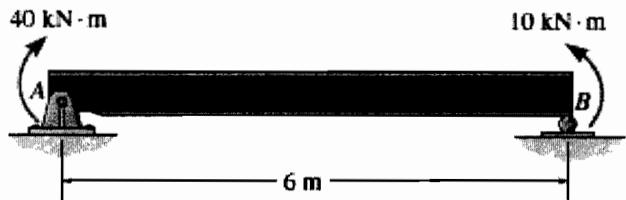
Problem # 1:

Given:

The beam shown

$$E = 200 \text{ GPa}$$

$$I = 39.9 \times 10^{16} \text{ m}^4$$



Required:

Maximum deflection

Solution:

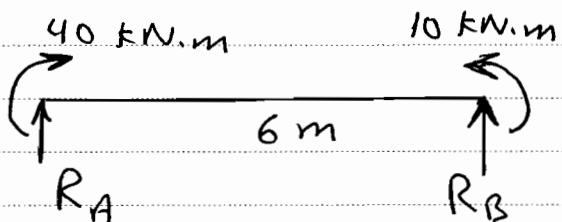
We will start with the moment equation.

We may start with the load equations.

We need to find

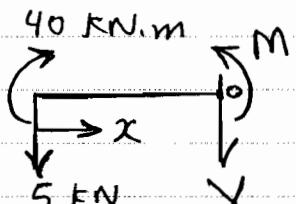
$$R_A. \text{ (Why?!)}$$

In FBD ①,



$$\begin{aligned} \rightarrow \sum M_B &= 0 \Rightarrow -40 + 10 - 6R_A = 0 & \text{FBD } ① \\ \Rightarrow R_A &= -5 \text{ kN} = 5 \text{ kN} \downarrow \end{aligned}$$

To get the moment equation,

we draw FBD ② by taking
a section "between A and B".Note that there is only one
moment equation. (How?!)

$$\rightarrow \sum M_0 = 0 \Rightarrow M = 40 - 5x = EI \frac{d^2v}{dx^2} \quad (\text{kN.m})$$

$$\text{Slope} = \theta = \frac{dv}{dx} = \int \frac{M}{EI} dx \Rightarrow$$

$$EI \frac{dv}{dx} = \int (40 - 5x) dx = 40x - \frac{5}{2}x^2 + C_1$$

$$EI v = \int EI \frac{dv}{dx} dx = \int EI (40x - \frac{5}{2}x^2 + C_1) dx \Rightarrow$$

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$$EIv = 20x^2 - \frac{5}{6}x^3 + C_1x + C_2$$

Boundary Conditions (B.C.s):

We have 2 B.C.s. and 2 unknowns (C_1 and C_2). Thus, we can find C_1 & C_2 .

$v(0) = 0$ [That is the deflection is zero at $x=0$ (A)] \Rightarrow

$$0 = 0 + 0 + 0 + C_2 \Rightarrow C_2 = 0$$

$$v(6) = 0 \Rightarrow 0 = 20(6)^2 - \frac{5}{6}(6)^3 + C_1(6) \Rightarrow C_1 = -90$$

"with appropriate units"!

Thus,

$$EIv = 20x^2 - \frac{5}{6}x^3 - 90x$$

To get v_{\max} , we set $\frac{dv}{dx} = 0 \Rightarrow$ get x . (Why?)

$$\Rightarrow \frac{dv}{dx} EI = 40x - \frac{5}{2}x^2 - 90 \equiv 0 \Rightarrow$$

$$2x^2 - 16x + 36 = 0 \Rightarrow x = 13.292 \text{ or } 2.7085_m$$

x (outside the range) ✓

$$\Rightarrow v_{\max} = \left[20(2.7085)^2 - \frac{5}{6}(2.7085)^3 - 90(2.7085) \right] / 200(10)^9(39.9)(10)^{-6} (10)^3$$

$$\Rightarrow v = -0.014236 \text{ m}$$

KN → N ↴

$$\Rightarrow v = 14.24 \text{ mm } \downarrow @ x = 2.709 \text{ m}$$

"Very small as in most applications"

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Problem #2:

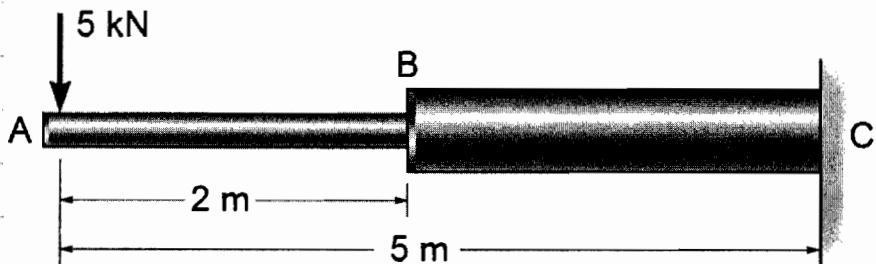
Given:

The beam shown

$$E = \text{constant}$$

$$I = I_0 \text{ for AB}$$

$$= 2I_0 \text{ for BC}$$



Required:

Deflection at B in terms of EI_0 .

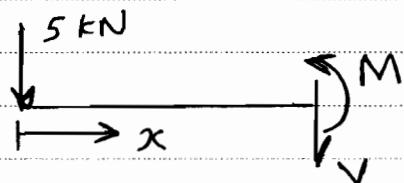
Solution:

We will start with the moment equation.

Note that there is only one moment equation for both AB and BC; however, there is a "discontinuity" in I at B. Thus, we need to divide the domain into two parts/segments (from A to B and from B to C) and use the "concept of continuity" in slope and deflection at B. \Rightarrow

From the FBD,

$$M = -5x \quad (\text{kN.m})$$

AB ($0 \leq x \leq 2$):

FBD

$$EI_0 \frac{d^2v}{dx^2} = M = -5x$$

$$EI_0 \frac{dv}{dx} = \int M dx = -\frac{5}{2}x^2 + C_1 \quad (1)$$

$$EI_0 v = \int (-\frac{5}{2}x^2 + C_1) dx = -\frac{5}{6}x^3 + C_1 x + C_2 \quad (2)$$

BC ($2 \leq x \leq 5$):

$$2EI_0 \frac{dv}{dx} = M = -5x \Rightarrow$$

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$$2EI_0 \frac{dv}{dx} = \int (-5x) dx = -\frac{5}{2}x^2 + C_3 \quad (3)$$

$$2EI_0 v = \int \left(-\frac{5}{2}x^2 + C_3\right) dx = -\frac{5}{6}x^3 + C_3x + C_4 \quad (4)$$

B.C.s :

$$v(5) = v_C = 0 \quad \text{on range BC} \quad [\text{eq } (4)]$$

$$\frac{dv}{dx}(5) = \theta_C = 0 \quad \text{on range BC} \quad [\text{eq. } (3)]$$

Continuity Conditions (C.C.s.) :

$$v_{AB} = v_{BC} \quad @ B$$

$$\left(\frac{dv}{dx}\right)_{AB} = \left(\frac{dv}{dx}\right)_{BC} \quad @ B$$

Now, we have 4 B.C.s & C.C.s. and 4 unknown constants. \approx We can find them.

$$v(5) = 0 \text{ in eq. (4)}, \Rightarrow -\frac{5}{6}(5)^3 + 5C_3 + C_4 = 0 \quad (5)$$

$$\frac{dv}{dx}(5) = 0 \text{ in eq. (3)}, \Rightarrow -\frac{5}{2}(5)^2 + C_3 = 0 \Rightarrow$$

$$C_3 = 62.5 \Rightarrow \text{into eq. (5)} \Rightarrow C_4 = -625/3$$

$$\left(\frac{dv}{dx}\right)(2)_{AB} = \left(\frac{dv}{dx}\right)(2)_{BC} \quad [\text{eq. (1) = eq. (3) @ B}]$$

$$\Rightarrow \frac{1}{EI_0} \left[-\frac{5}{2}(2)^2 + C_1 \right] = \frac{1}{2EI_0} \left[-\frac{5}{2}(2)^2 + 62.5 \right]$$

$$\Rightarrow C_1 = 36.25$$

$$v(2)_{AB} = v(2)_{BC} \quad [\text{eq. (2) = eq. (4) @ B}] \Rightarrow$$

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$$\frac{1}{EI_0} \left[-\frac{5}{6}(2)^3 + 36.25(2) + C_2 \right] = \frac{1}{2EI_0} \left[-\frac{5}{6}(2)^3 + 62.5(2) - \frac{625}{3} \right]$$

$$\Rightarrow C_2 = -665/6$$

$$v_B = v(2) \text{ on AB or on BC}$$

Use AB. \Rightarrow Eq. ② \Rightarrow

$$v_B = \left[-\frac{5}{6}(2)^3 + 36.25(2) - 665/6 \right] / EI_0 \Rightarrow$$

$$v_B = -45 / EI_0$$

check using eq. ④ [BC] :

$$v_B = \left[-\frac{5}{6}(2)^3 + 62.5(2) - 625/3 \right] / 2EI_0 \Rightarrow$$

$$v_B = -45 / EI_0 \quad \text{ok!}$$

Note that if we only need v_B , we can use eq. ④ after finding C_3 and C_4 from the B.C.s. Thus, no need for the C.C.s. to find C_1 and C_2 . In this case, we can not "check". However, if we need the eqs. of the elastic curves (def. eqs.), then we do as we did above.

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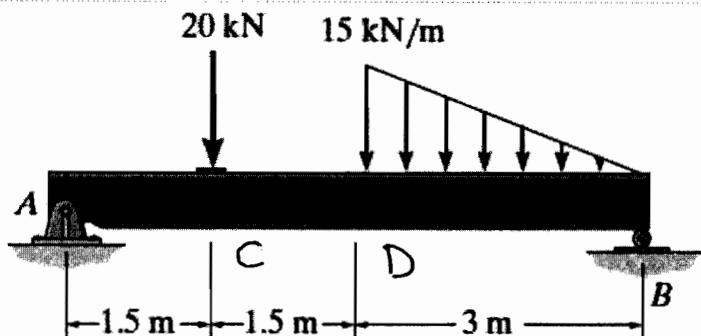
Problem # 3:

Given:

The beam shown

$$E = 200 \text{ GPa}$$

$$I = 65(10)^6 \text{ mm}^4$$



Required:

maximum deflection

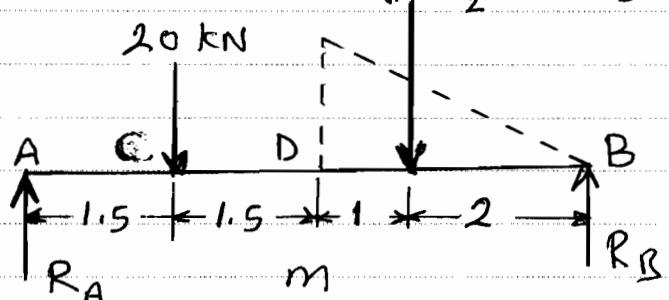
Solution:

Starting with the moment equation, we need R_A in FBD ①.

$$\sum M_B = 0 \Rightarrow$$

$$-6R_A + 20(4.5) + 22.5(2) = 0 \Rightarrow$$

$$R_A = 22.5 \text{ kN}$$



FBD ①

To get the moment equation, we make a section in the last segment and draw FBD ② for the left part.

$$w(x) = \frac{6-x}{3} \Rightarrow$$

$$w(x) = 5(6-x)$$

$$F_1 = 15(x-3)$$

$$F_2 = (15-w)\left(\frac{x-3}{2}\right) \uparrow$$

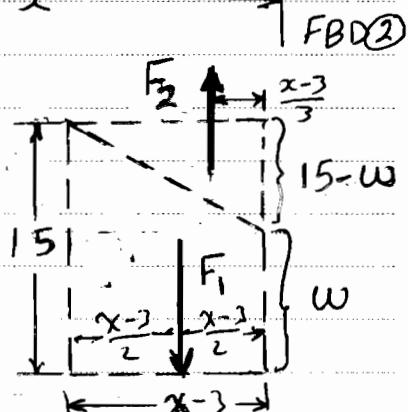
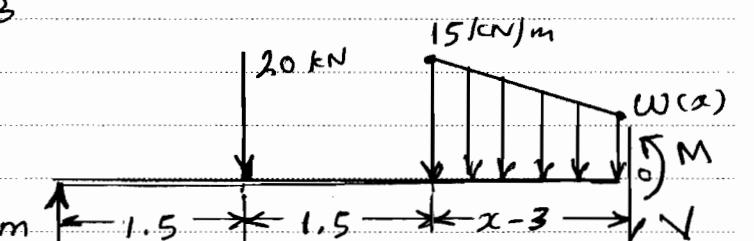
The locations of F_1 and F_2 are as shown.

$$F_2 = (15-w)\left(\frac{x-3}{2}\right)$$

$$= [15 - 5(6-x)](x-3)/2$$

$$= 5[(3-6+x)](x-3)/2$$

$$= \frac{5}{2}(x-3)^2$$



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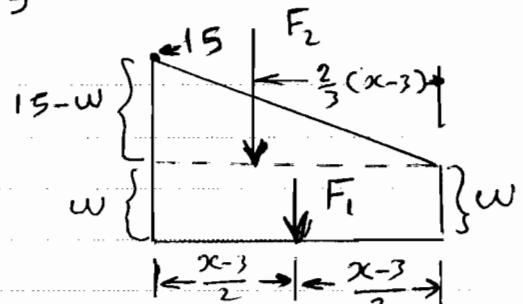
Try taking F_1 and F_2 as shown! \Rightarrow

What did you find?

$$\therefore \sum M_o = 0 \Rightarrow$$

$$M = 22.5x + 20(x-1.5)$$

$$+ 15(x-3)\left(\frac{x-3}{2}\right) - \frac{5}{2}(x-3)^2\left(\frac{x-3}{3}\right) = 0$$



$$F_1 = w(x-3) \downarrow$$

$$F_2 = (15-w)(x-3)/2 \downarrow$$

Moving the terms to the other side and using the singularity function form, we get:

$$M = 22.5 <x-0>^1 - 20 <x-1.5>^1 - \frac{15}{2} <x-3>^2 + \frac{5}{6} <x-3>^3$$

$$EI \frac{d^2v}{dx^2} = M \Rightarrow EI \frac{dv}{dx} = \int M dx \Rightarrow$$

$$EI \frac{dv}{dx} = 11.25 <x-0>^2 - 10 <x-1.5>^2 - \frac{15}{6} <x-3>^3 + \frac{5}{24} <x-3>^4 + C_1$$

$$EIv = 3.75 <x-0>^3 - \frac{10}{3} <x-1.5>^3 - \frac{15}{24} <x-3>^4 + \frac{1}{24} <x-3>^5 + C_1x + C_2$$

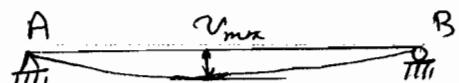
$$\text{B.C.s.: } v(0) = 0 \Rightarrow 0 - 0 - 0 + 0 + 0 + C_2 = 0 \Rightarrow C_2 = 0$$

Note that the value within $<x-a>$ can never be negative. If so, it is ZERO! (why and how?)

$$v(6) = v_B = 0 \Rightarrow 3.75(6)^3 - \frac{10}{3}(4.5)^3 - \frac{15}{24}(3)^4 + \frac{1}{24}(3)^5 + 6C_1 = 0$$

$$\Rightarrow C_1 = -77.625 \quad \text{units in kN.m}$$

$$v_{max} \text{ is } @ \frac{dv}{dx} = 0$$



(OR at the free end if there is.)

$$\Rightarrow \text{Set } \frac{dv}{dx} = 0 \Rightarrow 11.25x^2 - 10 <x-1.5>^2 - \frac{15}{6} <x-3>^3 + \frac{5}{24} <x-3>^4 - 77.625 = 0$$

We can either find the four roots of this 4th degree equation, or assume the root within our range ($0 \leq x \leq 6$)

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is between 0 and 3 m (as there is only one possible root within this range, as seen in the figure above). In this case, we have to check our assumption ($x \leq 3$). This will simplify our solution as the 3rd and 4th order terms are dropped. (How?!) \Rightarrow

$$11.25 x^2 - 10(x-1.5)^2 - 77.625 = 0 \Rightarrow$$

$$x = 2.96997 \text{ or } -2.96997$$

(ok)

"not possible"

$$\Rightarrow x_m = 2.970 \text{ m (within our range 0-3)} \Rightarrow \text{ok}$$

Thus v_{max} @ $x_m = 2.970 \text{ m} \Rightarrow$

$$v_{max} = \left[3.75 x_m^3 - \frac{10}{3} (x_m - 1.5)^3 - 77.625 x_m \right] (0) / 200(10)^9 \cdot 65(10)^6$$

\Rightarrow

$$v_{max} = -0.010992 \text{ m} = 10.99 \text{ mm} \downarrow$$

Again, very small compared with $L = 6 \text{ m}$

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Problem # 4:

Given:

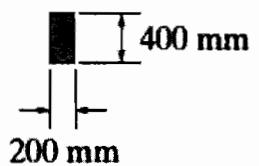
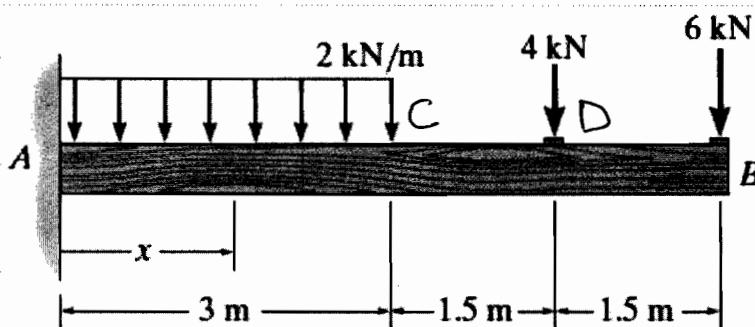
The beam shown

$$E = 12 \text{ GPa}$$

Required:

The eq. of the elastic curve (deflection)

Deflection & slope @ B



Solution:

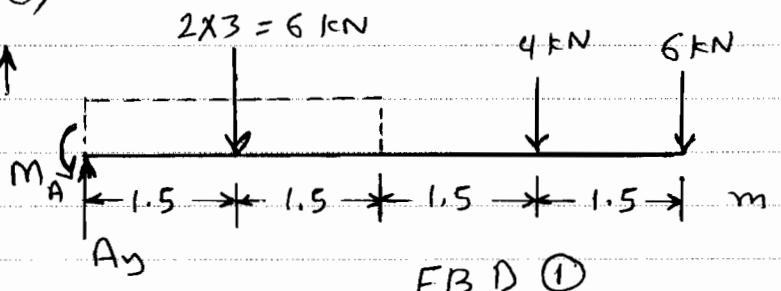
First, we need to find the reactions. From FBD ①,

$$\uparrow \sum F_y = 0 \Rightarrow A_y = 16 \text{ kN} \uparrow$$

$$\leftarrow \sum M_A = 0 \Rightarrow$$

$$M_A - 6(1.5) - 4(4.5) - 6(6) = 0$$

$$\Rightarrow M_A = 63 \text{ kN.m}$$

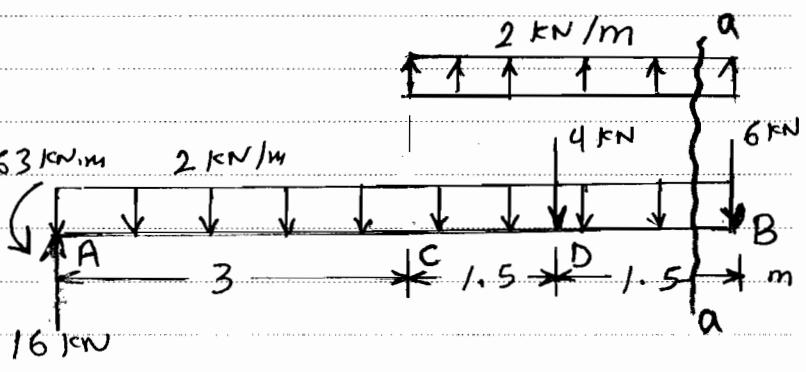


FB D ①

Using the singularity function, you need to remember that once the distributed load starts (from anywhere/x), it has to go all the way up to the end. (Why?)

Thus, the 2 kN/m - load has to go from A to B. Therefore we have to add an upward load equals to 2 kN/m from C to B, thus making the total loads equal to the original loading. This is shown in the figure below.

Now, we make a section (a-a) in the last segment (DB), and draw a FBD by taking



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the left part, as

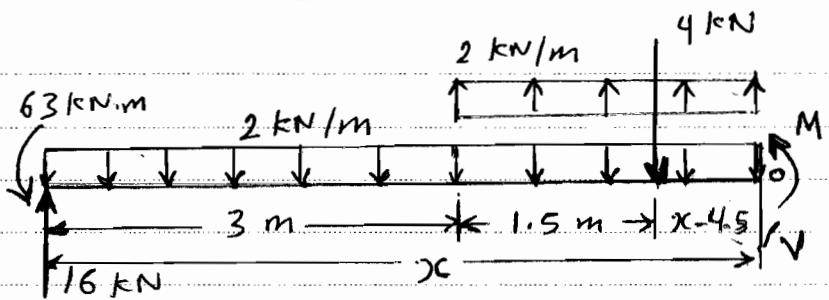
shown FBD ②.

Note that any concentrated load/moment

(whether applied

or reaction) does

not appear! Does it mean "it does not affect the beam"? NO!! HOW??!!



FBD ②

$\Rightarrow \sum M_o = 0 \Rightarrow$ In a singularity function form,

$$M = 16(x-0)' - 63(x-0)^0 - \frac{2}{2}(x-0)^2 + \frac{2}{2}(x-3)^2 - 4(x-4.5)'$$

$$EI \frac{dv}{dx} = 8(x-0)^2 - 63(x-0)' - \frac{1}{3}(x-0)^3 + \frac{1}{3}(x-3)^3 - 2(x-4.5)^2 + C_1$$

$$EI v = \frac{8}{3}(x-0)^3 - \frac{63}{2}(x-0)^2 - \frac{1}{12}(x-0)^4 + \frac{1}{12}(x-3)^4 - \frac{2}{3}(x-4.5)^3 + C_1 x + C_2$$

B.C.s.: $\frac{dv}{dx}$ and $v = 0$ @ $x = 0$ (A) \Rightarrow

$$\frac{dv}{dx}(0) = 0 - 0 - 0 + 0 - 0 + C_1 = 0 \Rightarrow C_1 = 0$$

$$v(0) = 0 = 0 - 0 - 0 + 0 - 0 + 0 + C_2 \Rightarrow C_2 = 0$$

Thus,

$$v = \frac{1}{EI} \left[\frac{8}{3}(x-0)^3 - \frac{63}{2}(x-0)^2 - \frac{1}{12}(x-0)^4 + \frac{1}{12}(x-3)^4 - \frac{2}{3}(x-4.5)^3 \right]$$

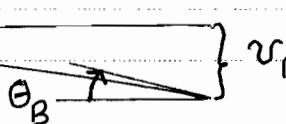
At B, $x = 6\text{ m}$; $E = 12(10)^9 \text{ N/m}^2$

$$I = bh^3/12 \Rightarrow I = (0.2)(0.4)^3/12 = \frac{3.2}{3}(10)^{-3} \text{ m}^4$$

$$\text{slope} = \theta_B = \frac{dv}{dx} = [8(6)^2 - 63(6) - \frac{1}{3}(6)^3 + \frac{1}{3}(3)^3 - 2(1.5)^2] (10)^3/EI$$

$$\Rightarrow \theta_B = -0.01230 \text{ rad} = -0.7050^\circ \quad \text{"CCW"}$$

$$v_B = \left[\frac{8}{3}(6)^3 - \frac{63}{2}(6)^2 - \frac{1}{12}(6)^4 + \frac{1}{12}(3)^4 - \frac{2}{3}(1.5)^3 \right] / EI \Rightarrow v_B = -0.05168 \text{ m}$$



not to scale

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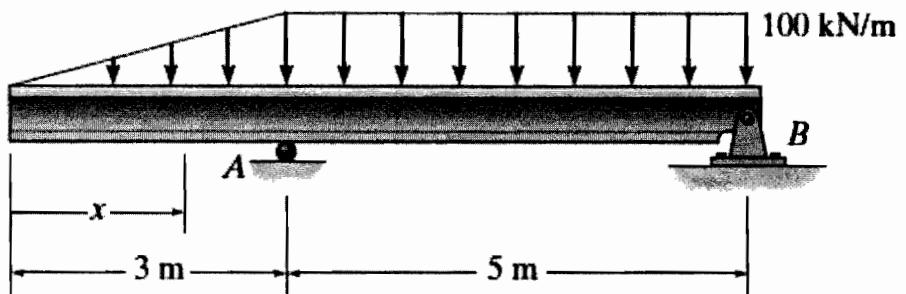
Problem # 5:

Given:

The beam

shown

$EI = \text{constant}$



Required:

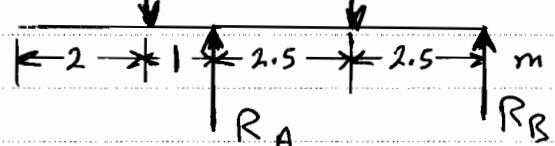
Eq. of elastic curve (deflection)

$$\frac{100(7)^2}{2} = 150 \text{ kN} \quad 100(5) = 500 \text{ kN}$$

Solution:

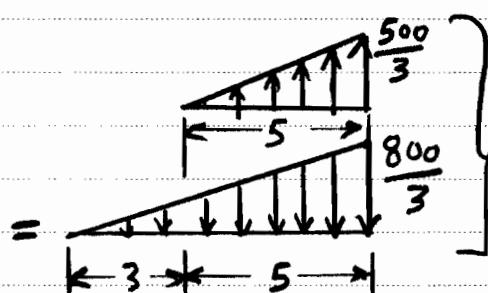
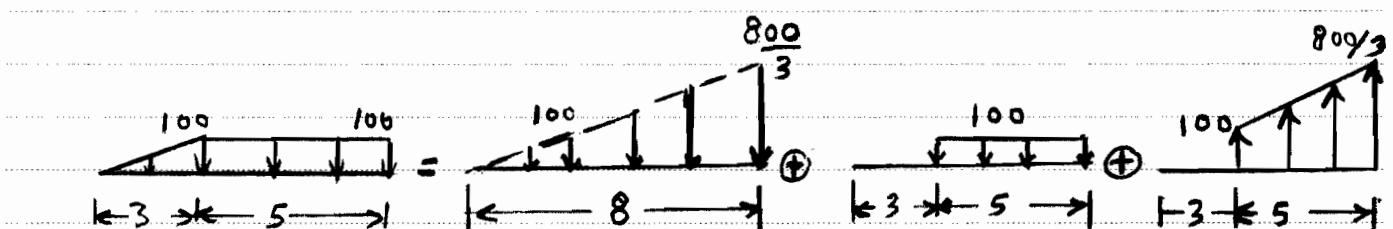
First, we need to determine R_A in FBD ①.

$$\therefore \sum M_B = 0 \Rightarrow 150(6) - 5R_A + 500(2.5) = 0 \Rightarrow R_A = 430 \text{ kN}$$

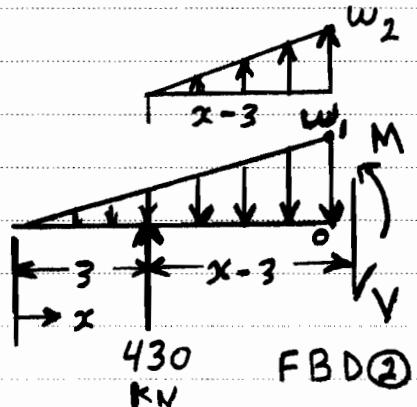


FBD ①

Since the distributed load has to continue to the end once it starts, we need to make equivalent load as shown below.



Now, a section is made in the last segment (between A and B), and FBD ② is drawn.



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$$\frac{w_1}{x} = \frac{800/3}{8} \Rightarrow w_1 = \frac{100}{3}x$$

$$\frac{w_2}{x-3} = \frac{500/3}{5} \Rightarrow w_2 = \frac{100}{3}(x-3)$$

$\leftarrow \sum M_o = 0 \Rightarrow -430(x-3) + \frac{100}{3}x\left(\frac{x}{2}\right)\left(\frac{x}{3}\right) - \frac{100}{3}(x-3)\left(\frac{x-3}{2}\right)\left(\frac{x-3}{3}\right) + M = 0$

Moving terms to the other side and expressing them in a singularity function form:

$$M = -\frac{100}{18} < x-0 >^3 + 430 < x-3 >^1 + \frac{100}{18} < x-3 >^3$$

$$EI \frac{dv}{dx} = \int M dx = -\frac{25}{18} < x-0 >^4 + 215 < x-3 >^2 + \frac{25}{18} < x-3 >^4 + C_1$$

$$EI v = -\frac{5}{18} < x-0 >^5 + \frac{215}{3} < x-3 >^3 + \frac{5}{18} < x-3 >^5 + C_1 x + C_2$$

B.C.'s: $v_A = v(3) = 0$ and $v_B = v(8) = 0$

Two equations and two constants: We can solve.

$$v(3) = 0 = -\frac{5}{18}(3)^5 + 3C_1 + C_2 \quad ①$$

$$v(8) = 0 = -\frac{5}{18}(8)^5 + \frac{215}{3}(5)^3 + \frac{5}{18}(5)^5 + 8C_1 + C_2 \quad ②$$

Solving eqs. ① and ② yields

$$C_1 = -\frac{475}{3} \text{ if } C_2 = \frac{1085}{2} \Rightarrow$$

$$v = \left[-\frac{5}{18} < x-0 >^5 + \frac{215}{3} < x-3 >^3 + \frac{5}{18} < x-3 >^5 - \frac{475}{3}x + \frac{1085}{2} \right] / EI$$

units in kN.m