HW#2 Key Solutions

•1-61. Determine the maximum magnitude P of the load the beam will support if the average shear stress in each pin is not to allowed to exceed 60 MPa. All pins are subjected to double shear as shown, and each has a diameter of 18 mm.

Referring to the FBD of member AB, Fig. a,

$$\zeta + \Sigma M_A = 0;$$
 $F_{BC} \sin 30^{\circ}(6) - P(2) - P(4) = 0$ $F_{BC} = 2P$
 $\xrightarrow{+} \Sigma F_x = 0;$ $A_x - 2P \cos 30^{\circ} = 0$ $A_x = 1.732P$
 $+ \uparrow \Sigma F_y = 0;$ $A_y - P - P + 2P \sin 30^{\circ} = 0$ $A_y = P$

Thus, the force acting on pin A is

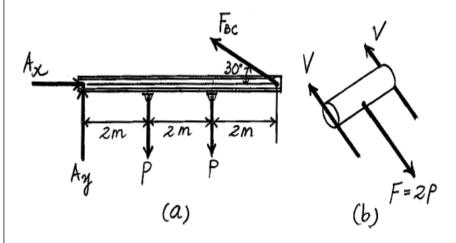
$$F_A = 2 \overline{A_x^2 + A_y^2} = 2 \overline{(1.732P)^2 + P^2} = 2P$$

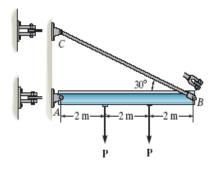
All pins are subjected to same force and double shear. Referring to the FBD of the pin, Fig. b,

$$V = \frac{F}{2} = \frac{2P}{2} = P$$

The cross-sectional area of the pin is $A = \frac{\pi}{4}(0.018^2) = 81.0(10^{-6})\pi$ m². Thus,

$$au_{\text{allow}} = \frac{V}{A};$$
 $60(10^6) = \frac{P}{81.0(10^{-6})\pi}$
$$P = 15\ 268\ \text{N} = 15.3\ \text{kN}$$
 Ans.





1-66. Determine the largest load P that can be a applied to the frame without causing either the average normal stress or the average shear stress at section a-a to exceed $\sigma = 150$ MPa and $\tau = 60$ MPa, respectively. Member CB has a square cross section of 25 mm on each side.

Analyse the equilibrium of joint C using the FBD Shown in Fig. a,

$$+\uparrow \Sigma F_y = 0;$$
 $F_{BC}\left(\frac{4}{5}\right) - P = 0$ $F_{BC} = 1.25P$

Referring to the FBD of the cut segment of member BC Fig. b.

$$\stackrel{+}{\to} \Sigma F_x = 0; \qquad N_{a-a} - 1.25P\left(\frac{3}{5}\right) = 0 \qquad \qquad N_{a-a} = 0.75P$$

$$N_{a-a} = 0.75P$$

$$+\uparrow\Sigma F_y=0; \qquad 1.25P\bigg(\frac{4}{5}\bigg)-V_{a-a}=0 \qquad \qquad V_{a-a}=P$$

$$V_{a-a}=P$$

The cross-sectional area of section a-a is $A_{a-a}=(0.025)\left(\frac{0.025}{3/5}\right)$ $= 1.0417(10^{-3})$ m². For Normal stress,

$$\sigma_{\text{allow}} = \frac{N_{a-a}}{A_{a-a}};$$
 $150(10^6) = \frac{0.75P}{1.0417(10^{-3})}$

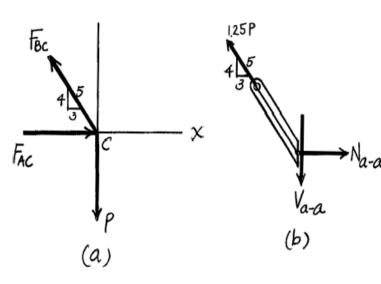
$$P = 208.33(10^3) \text{ N} = 208.33 \text{ kN}$$

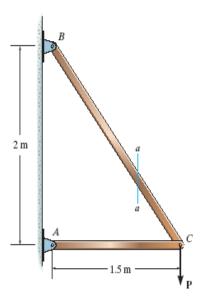
For Shear Stress

$$\tau_{\rm allow} = \frac{V_{a-a}}{A_{a-a}}; \qquad 60(10^6) = \frac{P}{1.0417(10^{-3})}$$

$$P = 62.5(10^3) \text{ N} = 62.5 \text{ kN (Controls!)}$$

Ans.





1-71. Determine the average normal stress at section a-a and the average shear stress at section b-b in member AB. The cross section is square, 0.5 in. on each side.

Consider the FBD of member BC, Fig. a,

$$\zeta + \Sigma M_C = 0$$
; $F_{AB} \sin 60^{\circ}(4) - 150(4)(2) = 0$ $F_{AB} = 346.41 \text{ lb}$

Referring to the FBD in Fig. b,

$$^{+} \swarrow \Sigma F_{x'} = 0;$$
 $N_{a-a} + 346.41 = 0$ $N_{a-a} = -346.41 \text{ lb}$

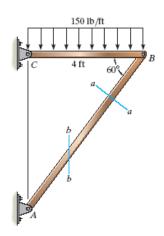
Referring to the FBD in Fig. c.

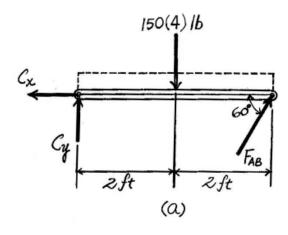
$$+\uparrow\Sigma F_{y}=0; \qquad V_{b-b}-346.41\sin 60^{\circ}=0 \qquad V_{b-b}=300~\mathrm{lb}$$

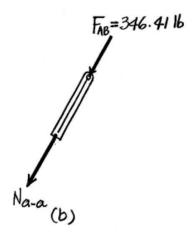
The cross-sectional areas of section a–a and b–b are $A_{a-a}=0.5(0.5)=0.25$ in 2 and $A_{b-b}=0.5\left(\frac{0.5}{\cos 60^\circ}\right)=0.5$ in 2 . Thus

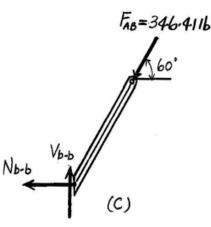
$$\sigma_{a-a} = \frac{N_{a-a}}{A_{a-a}} = \frac{346.41}{0.25} = 1385.64 \,\mathrm{psi} = 1.39 \,\mathrm{ksi}$$
 Ans.

$$au_{b-b} = \frac{V_{b-b}}{A_{b-b}} = \frac{300}{0.5} = 600 \text{ psi}$$
 Ans.

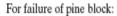








1-87. The 60 mm × 60 mm oak post is supported on the pine block. If the allowable bearing stresses for these materials are $\sigma_{\text{oak}} = 43 \text{ MPa}$ and $\sigma_{\text{pine}} = 25 \text{ MPa}$, determine the greatest load P that can be supported. If a rigid bearing plate is used between these materials, determine its required area so that the maximum load P can be supported. What is this load?



$$\sigma = \frac{P}{A}$$
, $25(10^6) = \frac{P}{(0.06)(0.06)}$
 $P = 90 \text{ kN}$



Ans.

Ans.

For failure of oak post:

$$\sigma = \frac{P}{A}$$
; $43(10^6) = \frac{P}{(0.06)(0.06)}$
 $P = 154.8 \text{ kN}$

Area of plate based on strength of pine block:

$$\sigma = \frac{P}{A}$$
, $25(10^6) = \frac{154.8(10)^3}{A}$
 $A = 6.19(10^{-3})\text{m}^2$ Ans.
 $P_{max} = 155 \text{ kN}$ Ans.

1–94. If the allowable shear stress for each of the 0.30-in.-diameter steel pins at A, B, and C is $\tau_{\rm allow}=12.5$ ksi, and the allowable normal stress for the 0.40-in.-diameter rod is $\sigma_{\rm allow}=22$ ksi, determine the largest intensity w of the uniform distributed load that can be suspended from the beam.

Assume failure of pins B and C:

$$\tau_{\text{allow}} = 12.5 = \frac{1.667w}{\frac{\pi}{4}(0.3^2)}$$

$$w = 0.530 \text{ kip/ft}$$
 (controls)

Assume failure of pins A:

$$F_A = 2 \overline{(2w)^2 + (1.333w)^2} = 2.404 w$$

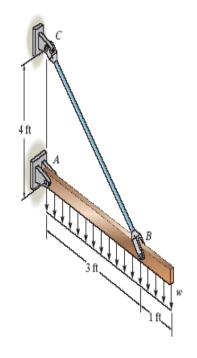
$$\tau_{\text{allow}} = 12.5 = \frac{1.202w}{\frac{\pi}{4}(0.3^2)}$$

$$w = 0.735 \,\mathrm{kip/ft}$$

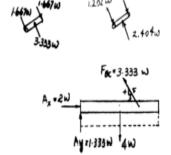
Assume failure of rod BC:

$$\sigma_{\text{allow}} = 22 = \frac{3.333w}{\frac{\pi}{4}(0.4^2)}$$

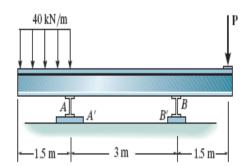
$$w = 0.829 \text{ kip/ft}$$



Ans.



*1–96. If the allowable bearing stress for the material under the supports at A and B is $(\sigma_b)_{\text{allow}} = 1.5$ MPa, determine the maximum load P that can be applied to the beam. The bearing plates A' and B' have square cross sections of $150 \text{ mm} \times 150 \text{ mm}$ and $250 \text{ mm} \times 250 \text{ mm}$, respectively.



Referring to the FBD of the beam, Fig. a,

$$\zeta + \Sigma M_A = 0; \qquad N_B(3) \, + \, 40(1.5)(0.75) \, - \, P(4.5) = 0 \qquad N_B = 1.5P \, - \, 15$$

$$\zeta + \Sigma M_B = 0;$$
 $40(1.5)(3.75) - P(1.5) - N_A(3) = 0$ $N_A = 75 - 0.5P$

For plate A',

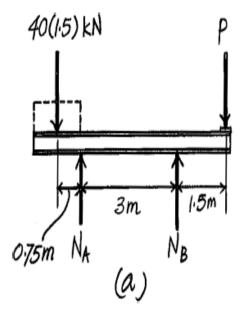
$$(\sigma_b)_{\text{allow}} = \frac{N_A}{A_{A'}};$$
 $1.5(10^6) = \frac{(75 - 0.5P)(10^3)}{0.15(0.15)}$

$$P = 82.5 \, \text{kN}$$

For plate B',

$$(\sigma_b)_{\text{allow}} = \frac{N_B}{A_{B'}};$$
 $1.5(10^6) = \frac{(1.5P - 15)(10^3)}{0.25(0.25)}$

$$P = 72.5 \text{ kN}$$
 (Controls!)



Ans.

1–103. Determine the required thickness of member BC and the diameter of the pins at A and B if the allowable normal stress for member BC is $\sigma_{\rm allow}=29~{\rm ksi}$ and the allowable shear stress for the pins is $\tau_{\rm allow}=10~{\rm ksi}$.

Referring to the FBD of member AB, Fig. a,

$$\zeta + \Sigma M_A = 0;$$
 $2(8)(4) - F_{BC} \sin 60^{\circ} (8) = 0$ $F_{BC} = 9.238 \text{ kip}$

$$\stackrel{+}{\to} \Sigma F_x = 0;$$
 9.238 cos 60° - $A_x = 0$ $A_x = 4.619 \text{ kip}$

$$+\uparrow \Sigma F_y = 0;$$
 9.238 sin 60° - 2(8) + $A_y = 0$ $A_y = 8.00$ kip

Thus, the force acting on pin A is

$$F_A = 2 \overline{A_x^2 + A_y^2} = 2 \overline{4.619^2 + 8.00^2} = 9.238 \text{ kip}$$

Pin A is subjected to single shear, Fig. c, while pin B is subjected to double shear, Fig. b.

$$V_A = F_A = 9.238 \text{ kip}$$
 $V_B = \frac{F_{BC}}{2} = \frac{9.238}{2} = 4.619 \text{ kip}$

For member BC

For member BC
$$\sigma_{\text{allow}} = \frac{F_{BC}}{A_{BC}}; \qquad 29 = \frac{9.238}{1.5(t)} \qquad t = 0.2124 \text{ in.}$$

Use
$$t = \frac{1}{4}$$
 in. Ans.

For pin A,

$$\tau_{\text{allow}} = \frac{V_A}{A_A};$$
 $10 = \frac{9.238}{\frac{\pi}{4}d_A^2}$ $d_A = 1.085 \text{ in.}$

Use
$$d_A = 1\frac{1}{8}$$
 in Ans.

For pin B,

$$au_{\text{allow}} = \frac{V_B}{A_B}; \qquad 10 = \frac{4.619}{\frac{\pi}{4}d_B^2} \qquad d_B = 0.7669 \text{ in}$$

Use
$$d_B = \frac{13}{16}$$
 in

Ans.

2 kip/ft

