

CE 203 (112)  
**KEY SOLUTION**  
EXAM 1

<b>PROBLEM</b>	<b>FACULTY</b>
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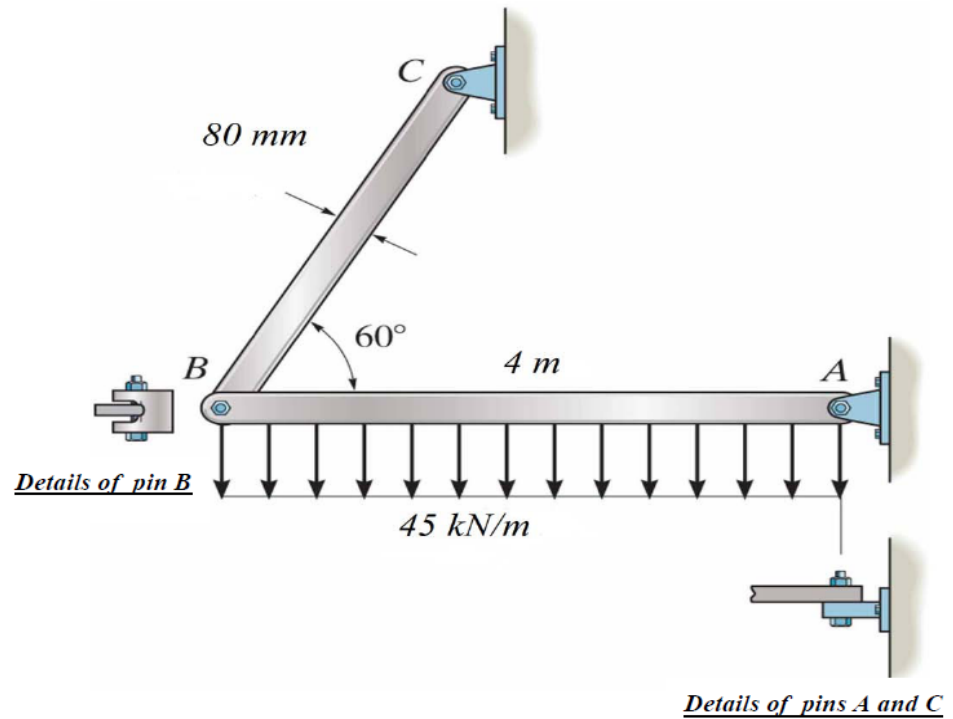
## Problem # 1

The beam is supported by a pin at  $A$  and link  $BC$ . Determine:

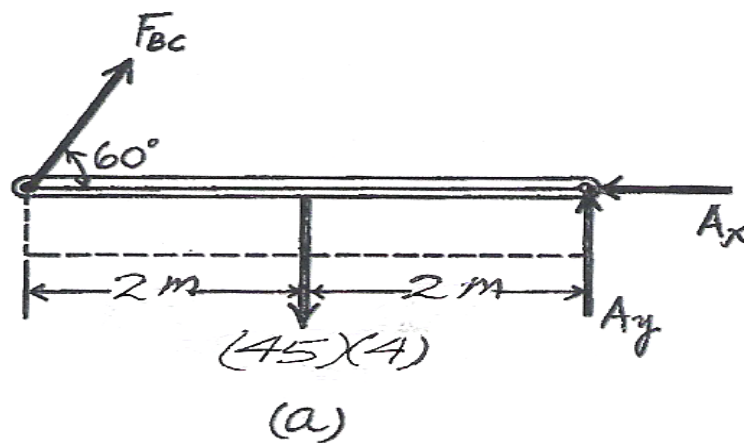
- the average shear stresses in the pins at  $A$  and  $B$ ,
- the average normal stress in link  $BC$ ,
- the bearing stress between pin  $C$  and the link.

**All pins have a diameter of 20 mm**

**Thickness of link  $BC = 30$  mm**



**Solution:**



Referring to the FBD of member AB in Figure (a),

$$\curvearrowleft + \sum M_A = 0; \quad (45 \text{ kN/m} \times 4 \text{ m})(4 \text{ m}/2) - (F_{BC} \sin 60^\circ)(4 \text{ m}) = 0,$$

$$F_{BC} = 90 / \sin 60^\circ = \underline{103.923 \text{ kN}}$$

$$\rightarrow + \sum F_x = 0; \quad -A_x + F_{BC} \cos 60^\circ = 0,$$

$$A_x = \underline{51.962 \text{ kN}}$$

$$\uparrow + \sum F_y = 0; \quad A_y + F_{BC} \cos 60^\circ - (45 \text{ kN/m})(4 \text{ m}) = 0,$$

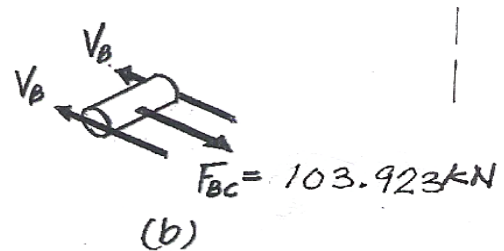
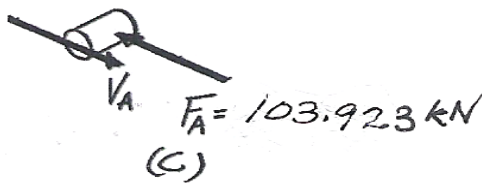
$$A_y = \underline{90.000 \text{ kN}}$$

Thus the force acting on pin A is

$$F_A = \sqrt{(A_x)^2 + (A_y)^2} = \sqrt{(51.962 \text{ kN})^2 + (90.000 \text{ kN})^2}$$

$$F_A = \underline{103.923 \text{ kN}}$$

Pin A is subjected to *single shear*, Figure (b), while pin B is subjected to double shear, Figure (c),



$$V_A = F_A = \underline{103.923 \text{ kN}}, \quad \text{while} \quad V_B = F_{BC} / 2 = 103.923 / 2 = \underline{51.962 \text{ kN}}$$

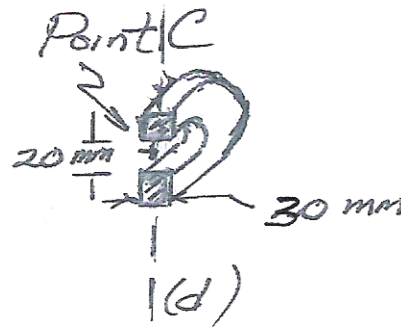
$$(\tau_A)_{\text{average}} = V_A / A = 103.923 \text{ kN} / (\pi \times 0.01^2) \text{ m}^2 = \underline{330.797 \text{ MPa}}$$

$$(\tau_B)_{\text{average}} = V_B / A = 51.962 \text{ kN} / (\pi \times 0.01^2) \text{ m}^2 = \underline{165.400 \text{ MPa}}$$

Cross sectional area of the link  $BC$  is  $30\text{mm} \times 80\text{mm} = 2400\text{mm}^2 = 2.4 \times 10^{-3}\text{m}^2$

$$(\sigma_{BC})_{\text{average}} = F_{BC} / A = 103.923 \text{ kN} / 2.4 \times 10^{-3}\text{m}^2 = \underline{43.301 \text{ MPa}}$$

Referring to the FBD of a section through point C in link BC, Figure (d),



$$F_C = F_{BC} = 103.923 \text{ kN}$$

$$A_{\text{bearing}} = 30\text{mm} \times 20\text{mm} = 600\text{mm}^2 = 0.6 \times 10^{-3}\text{m}^2$$

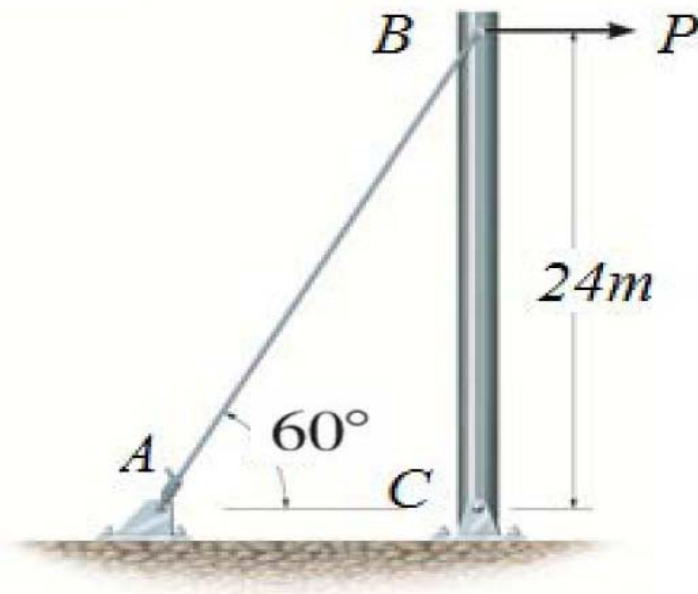
$$(\sigma_{\text{bearing}})_{BC} = F_C / A_{\text{bearing}} = 103.923 \text{ kN} / 0.6 \times 10^{-3}\text{m}^2 = \underline{173.205 \text{ MPa}}$$

## Problem # 2

The rigid pipe is supported by a pin at  $C$  and wire  $AB$ . The pin has a diameter of  $20$  mm while the wire has a diameter of  $10$  mm. If the allowable normal stress for the wire is  $\sigma_{allow} = 255$  MPa and the allowable shear stress for the pin is  $\tau_{allow} = 131$  MPa, determine:

- the maximum  $P$  that can be applied to the assembly,
- the increase in length and reduction in diameter of wire  $AB$ .

Use  $E = 70$  GPa and  $\nu = 0.35$ .

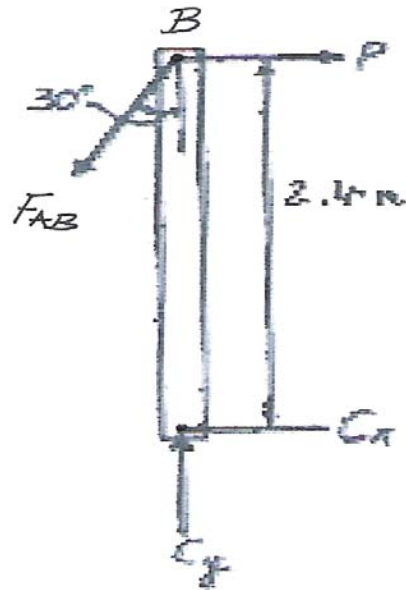


Detail of connection at C

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### *Solution:*

Referring to the FBD of the rigid pipe BC, Figure below;



$$\curvearrowright + \sum M_A = 0; \quad (F_{AB} \sin 30^\circ)(2.4\text{m}) - P(2.4\text{m}) = 0,$$

$$F_{AB} = P / \sin 30^\circ = \underline{2P}$$

$$\rightarrow + \sum F_x = 0; \quad - F_{AB} \sin 30^\circ + P - C_x = 0,$$

$$C_x = P - F_{AB} \sin 30^\circ$$

$$C_x = P - (P / \sin 30^\circ) \sin 30^\circ = \underline{0}$$

$$+\uparrow \sum F_y = 0;$$

$$C_y - F_{AB} \cos 30^\circ = 0,$$

$$C_y - (P / \sin 30^\circ) \cos 30^\circ = 0$$

$$C_y = \underline{1.732 P}$$

The stress in the guy wire  $\sigma_{AB} = \sigma_{allow} = 255 \text{ MPa} = F_{AB} / A_{\text{guy wire}} = 2P / \pi(0.005\text{m})^2$

$$P = (255 \times 10^6 \text{ N/m}^2) [\pi(0.005\text{m})^2] / 2$$

$$= \underline{10.014 \text{ kN}}$$

Pin  $C$  is subjected to a *double-shear* action by the reaction at  $C$ ;

$$F_C = C_y/2 = 1.732P/2$$

$$\tau_{\text{pin C}} = \tau_{\text{allow}} = 131 \text{ MPa} = F_C / A_{\text{pin C}} = 1.732P/2[\pi(0.01\text{m})^2]$$

$$P = (131 \times 10^6 \text{ N/m}^2)(2)[\pi(0.01\text{m})^2] / 1.732$$

$$= \underline{47.523 \text{ kN}}$$

Therefore,  $P_{\text{max}}$  should not exceed  $10.014 \text{ kN}$

$$P_{\text{max}} = \underline{10.014 \text{ kN}}$$

The axial force in the guy wire  $AB$  is  $F_{AB} = 2P = 20.028 \text{ kN}$ , but  $\sigma_{AB} = E \epsilon_{AB}$ , where  $\epsilon_{AB} = \Delta_{AB} / L_{AB}$ , and  $\sigma_{AB} = F_{AB} / A_{\text{guy wire}}$  or

$$F_{AB} / A_{\text{guy wire}} = E(\Delta_{AB} / L_{AB})$$

The stretch in the guy wire  $AB$  is

$$\Delta_{AB} = (F_{AB} L_{AB} / A_{\text{guy wire}} E) = [2P / (A_{\text{guy wire}} \times E)] L_{AB}$$

$$\Delta_{AB} = \{20.028\text{kN} / [\pi(0.005\text{m})^2 \times 70 \times (10)^6 \text{kN/m}^2]\} (2.4\text{m} / \cos 30^\circ)$$

$$\underline{\Delta_{AB} = 10.42 \times 10^{-3} \text{ m}}$$

Poisson's Ratio;  $\nu = \epsilon_{\text{Radial}} / \epsilon_{AB} = 0.35$

$$\epsilon_{\text{Radial}} = -0.35 \times \epsilon_{AB} = 0.35 \times 3.643 \times 10^{-3} = -1.2754 \times 10^{-3}$$

The reduction in the diameter is therefore.

$$\Delta_{\text{Radia}} = (-1.275 \times 10^{-3}) (10\text{mm}) = -12.75 \times 10^{-3} \text{ mm}$$

### Problem # 3

### Key Solution:

Rod ABC has a negligible mass and only supports two axial loads  $P$  and  $21P$  as shown. If *only* part AB is subjected to a temperature change  $\Delta T = 40^\circ\text{C}$ ; determine:

- the required value of  $P$  if the total length ABC remains constant,
- the displacement  $\delta_B$  of point B,
- the relative displacement  $\delta_{B/C}$ ,
- the final length of rod AB.

Given  $E = 70 \text{ GPa}$  and  $\alpha = 24 \times 10^{-6}/^\circ\text{C}$ .

$$N_1 = +P \quad (\uparrow) \quad ; \quad N_2 = A_y \quad ; \quad P - 21P - A_y = 0 \Rightarrow N_2 = 20P \quad (C)$$

(8)

$$a) \quad (\Delta l)_{AB}^{tot} + (\Delta l)_{BC}^{tot} = 0$$

$$N_1 l_1 / EA_1 + N_2 l_2 / EA_2 + \alpha l_2 \Delta T = 0$$

$$\frac{P(0.40)}{A_1} - \frac{20P(0.60)}{A_2} + \alpha E l_2 \Delta T = 0$$

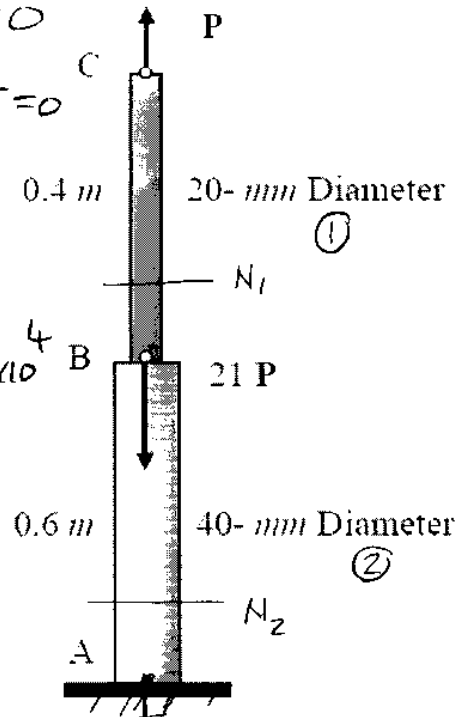
$$A_1 = 3.1416 \times 10^{-4} \text{ m}^2$$

$$A_2 = 1.2566 \times 10^{-3} \text{ m}^2$$

$$\alpha E l_2 \Delta T = 4.032 \times 10^7 \text{ N/m}$$

$$1.2732 \times 10^3 P - 9.5496 \times 10^3 P = -4.032 \times 10^4$$

$$\therefore P = 4.872 \text{ kN}$$



(5)

$$b) \quad \delta_B = \delta_A + (\delta_{B/A})^{tot}$$

$$= 0 + \alpha l \Delta T + \frac{N_2 l_2}{A_2 E}$$

$$= 5.76 \times 10^{-4} - \frac{20 \times 4.872 \times 0.6}{1.2566 \times 10^{-3} \times 70 \times 10^6}$$

$$= 5.76 \times 10^{-4} - 6.6465 \times 10^{-4}$$

$$= -8.8651 \times 10^{-5} \text{ m}$$

$$\Rightarrow \therefore \delta_B \approx 0.0887 \text{ mm} \quad (\downarrow)$$

(4)

$$c) \quad \delta_{B/C} = (\Delta l)_{BC}^{tot} = N_1 l_1 / A_1 E = \frac{1.2732 \times 10^3 P}{E}$$

$$= 8.862 \times 10^{-5} \text{ m}$$

(3)

$$d) \quad (L_{AB})_f = (L_{AB})_0 + (\Delta l)_{AB}^{tot} = 0.6 + \delta_{B/A}$$

$$= 0.59991 \text{ m}$$

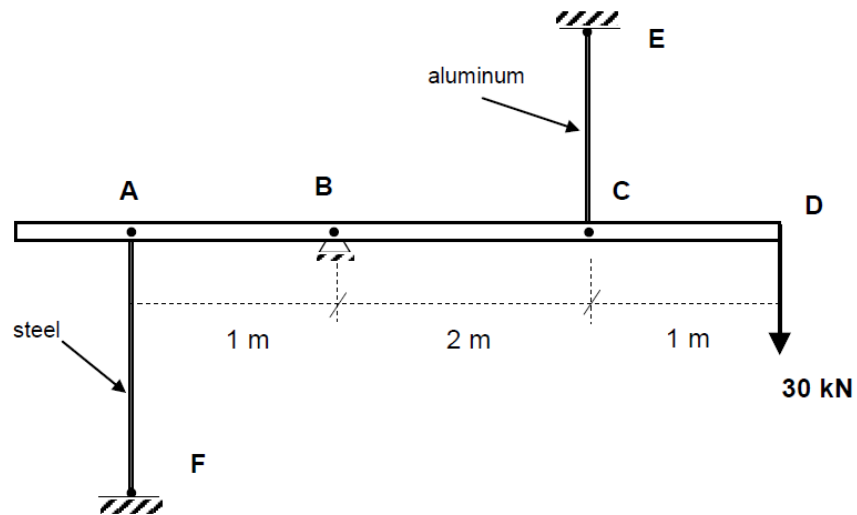


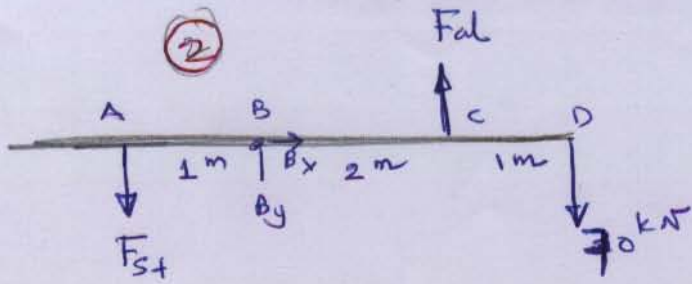
## Problem # 4

The rigid member  $ABCD$  is supported by a pin at B and two cables.

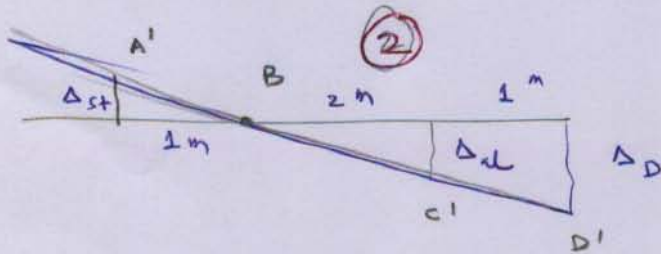
- Calculate the stresses in the cables due to the application of the shown force.
- Calculate the vertical displacement of point D.
- In one **sentence**, explain what will happen to the stresses in the cables if the temperature of the steel cable only is increased.

Cable	Length	Area ( $\text{mm}^2$ )	Material	E (GPa)
AF	1.5 m	315	Steel	200
CE	1 m	600	Aluminum	70





FBD ABCD



Geometry after deformation

Displacement of point D

using figure

$$\frac{\Delta_D}{3} = \frac{\Delta_{al}}{2} \quad (1)$$

$$\begin{aligned} \therefore \Delta_D &= 1.5 \Delta_{al} \\ &= 1.5 \frac{F_{al} L_{al}}{A_{al} E_{al}} \\ &= 1.5 \frac{(84)(1000)}{(600)(70)} \\ \Delta_D &= 3 \text{ mm} \end{aligned} \quad (2)$$

If steel temp is increased

$\sigma_{\text{steel}}$  will decrease (2)  
 $\sigma_{\text{Alum}}$  will increase

	L	A
steel	1.5	315
al	1	600

$$\sum M_B = 0$$

$$-F_{st}(1) - F_{al}(2) + 70(3) = 0$$

$$F_{st} + 2 F_{al} = 210 \quad (1)$$

(statically indeterminate)  
 Compatibility (2)

$$\frac{\Delta_{st}}{1} = \frac{\Delta_{al}}{2}$$

$$\Delta_{al} = 2 \Delta_{st} \quad (2)$$

put in terms of forces

$$\frac{F_{al} L_{al}}{A_{al} E_{al}} = 2 \frac{F_{st} L_{st}}{A_{st} E_{st}}$$

$$F_{al} = 2 F_{st} \quad (2)$$

Solving (1) & (2) we get

$$F_{st} = 42 \text{ kN} \quad ; \quad F_{al} = 84 \text{ kN}$$

$$\sigma_{st} = \frac{F_{st}}{A_{st}} = \frac{42 \times 10^3}{315} = 133 \text{ MPa} \quad (1)$$

$$\sigma_{al} = \frac{84 \times 10^3}{600} = 140 \text{ MPa} \quad (1)$$

Block **A** rests on block **B** as shown. Each block is a cube with initial dimensions 200x200x200 mm. The 4 side-faces of block **A** are free to displace, while the 4 side-faces of block **B** are prevented from expanding (i.e. restrained in the  $x$  and  $y$  directions). Determine:

- the vertical displacement of the force  $F$ ,
- the stress  $\sigma_x$  for block **A** and for block **B**,
- the value of the Shear Modulus ( $G$ ) for block **A**.

Ignore self-weight and any friction.

$$E = 10 \text{ GPa}, \text{ and } \nu = 0.2$$

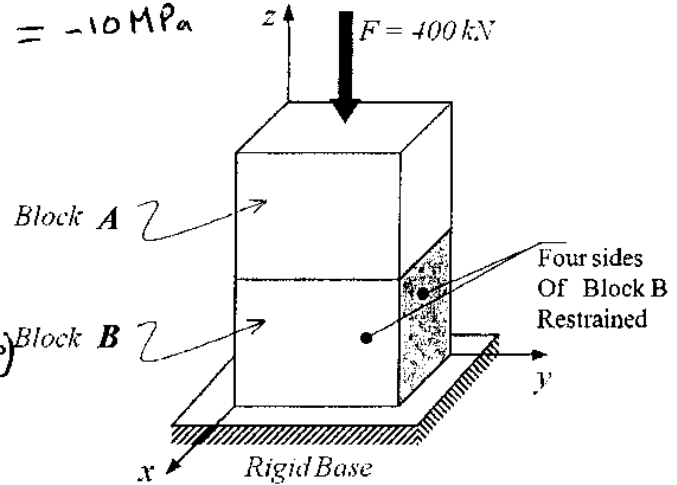
a)

$$\text{For block A } \sigma_z = \frac{-400,000}{(200)(200)} = -10 \text{ MPa}$$

sides are free, so can be considered an axial rod

$$(\Delta h)_A = \frac{Nl}{EA} = \frac{(-400,000)(200)}{(10,000)(200)(200)}$$

$$(\Delta h)_A = -0.2 \text{ mm}$$



For block **B**, 4 sides are restrained,

$$\sigma_z = -10 \text{ MPa}, \sigma_x = ?, \sigma_y = ?, \epsilon_x = \epsilon_y = 0, \epsilon_z = ?$$

$$\begin{aligned} \text{using } \epsilon_x = 0 &\rightarrow \sigma_x - 2\sigma_y - (-2)(-10) = 0 \\ \text{using } \epsilon_y = 0 &\rightarrow \sigma_y - 2\sigma_x - (-2)(-10) = 0 \end{aligned} \quad \left. \vphantom{\begin{aligned} \text{using } \epsilon_x = 0 \\ \text{using } \epsilon_y = 0 \end{aligned}} \right\} \text{ solve together}$$

$$\text{For block B } \sigma_x = \sigma_y = -2.5 \text{ MPa}, \epsilon_z = \frac{1}{10,000} [-10 + (2)(-5)]$$

$$\epsilon_z = -9 \times 10^{-4}, \rightarrow \text{so } (\Delta h)_B = (\epsilon_z)(200) = -0.18 \text{ mm}$$

$$\text{Displacement of Force} = (\Delta h)_A + (\Delta h)_B = -0.2 - 0.18 = -0.38 = \boxed{0.38 \text{ mm} \downarrow}$$

b) For Block **A**:  $\sigma_x = 0$  (free sides), Block **B**:  $\sigma_x = 2.5 \text{ MPa}$  (c) (from above)

$$\text{c) } G = \frac{E}{2(1+\nu)} = \frac{10 \text{ GPa}}{2(1+0.2)} = \boxed{4.167 \text{ GPa}} \quad \underline{\underline{\text{Ans}}}$$