Transformation of Stress
Theory \& Examples

* Triaxial states of stress are shown in Fig. (1).

Only positive stresses on positive faces are shown

* Biaxial states of stress (Plane Stress):

When all stresses act in the same plane.
$\Rightarrow$ Work in 2-D as shown in Fig. (2) in the $\underline{x-y}$ directions.


It is possible to have the orientation in different directions,
Fig. (1) as shown in Fig. (3) in the $\underline{x}^{\prime}-y^{\prime}$ directions.

Now, relationships between the stresses in the $x-y$ and $x^{\prime}-y^{\prime}$ directions are sought.



From Fig. (4), the sum of forces in the $\boldsymbol{x}^{\prime}$ and $\boldsymbol{y}^{\prime}$ directions must be zero.

* Note that forces not stresses are added.
(2 eqs. \& 2 unkns.)
$\sum F_{x^{\prime}}=0 \& \sum F_{y^{\prime}}=0 \Rightarrow$
$\sigma_{x^{\prime}}=\sigma_{x} \cos ^{2} \theta+\sigma_{y} \sin ^{2} \theta+2 \tau_{x y} \sin \theta \cos \theta$

$\tau_{x} y^{\prime}=-\left(\sigma_{x}-\sigma_{y}\right) \sin \theta \cos \theta+\tau_{x y}\left(\cos ^{2} \theta-\sin ^{2} \theta\right)$

Recall that
Fig. (4)

$$
\begin{align*}
& \cos ^{2} \theta=(1+\cos 2 \theta) / 2 \\
& \sin ^{2} \theta=(1-\cos 2 \theta) / 2 \\
& \sin \theta \cos \theta=\sin 2 \theta / 2 \\
& \cos ^{2} \theta-\sin ^{2} \theta=\cos 2 \theta \\
& \sigma_{x^{\prime}}=\frac{\sigma_{x}+\sigma_{y}}{2}+\frac{\sigma_{x}-\sigma_{y}}{2} \cos 2 \theta+\tau_{x y} \sin 2 \theta  \tag{1}\\
& \tau_{x^{\prime} y^{\prime}}=-\frac{\sigma_{x}-\sigma_{y}}{2} \sin 2 \theta+\tau_{x y} \cos 2 \theta \tag{2}
\end{align*}
$$

Similarly,

$$
\begin{align*}
& \sigma_{y^{\prime}}=\frac{\sigma_{x}+\sigma_{y}}{2}-\frac{\sigma_{x}-\sigma_{y}}{2} \cos 2 \theta-\tau_{x y} \sin 2 \theta  \tag{3}\\
& \tau_{y^{\prime} x^{\prime}}=\tau_{x^{\prime} y^{\prime}} \tag{4}
\end{align*}
$$

* Note that $\sigma_{x}+\sigma_{y}=\sigma_{x^{\prime}}+\sigma_{y^{\prime}}=I($ Invariant with any $\underline{\underline{\theta}}) \quad \Leftarrow$ Use it to check.

When $\sigma \& \tau$ on any two orthogonal faces are known, the stress components on all (any) faces (plane stress) can be calculated.

## Principal Normal Stresses:

$\sigma_{x^{\prime}}=f(\theta) \Rightarrow$ to get $\sigma_{x^{\prime} \max }$, set $\frac{\mathrm{d} \sigma_{x^{\prime}}}{\mathrm{d} \theta}=0 \quad \Rightarrow \quad$ find $\theta_{p} \quad \Rightarrow \sigma_{x^{\prime} \max }$
$\frac{d \sigma_{x^{\prime}}}{d \theta}=-\left(\sigma_{x}-\sigma_{y}\right) \sin 2 \theta_{\rho}+2 \tau_{x y} \cos 2 \theta_{\rho}=0$

Dividing by $\cos 2 \theta$,

$$
\begin{equation*}
\tan 2 \theta_{p}=\frac{2 \tau_{x y}}{\sigma_{x}-\sigma_{y}}=\frac{\tau_{x y}}{\left(\sigma_{x}-\sigma_{y}\right) / 2} \tag{5}
\end{equation*}
$$

From the equation above, Fig. (5) shown can be constructed.
$R=\sqrt{\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)^{2}+\tau_{x y}^{2}}$

$\cos 2 \theta_{p 1}=\frac{\left(\sigma_{x}-\sigma_{y}\right) / 2}{R}$
$\sin 2 \theta_{\rho 2}=\frac{-\tau_{x y}}{R} \quad ; \quad \cos 2 \theta_{\rho 2}=\frac{\left(\sigma_{x}-\sigma_{y}\right) / 2}{R}$
$\Rightarrow \sqrt{\sigma_{\max }=\frac{\sigma_{x}+\sigma_{y}}{2} \pm \sqrt{\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)^{2}+\tau_{x y}^{2}}}=\frac{\sigma_{x}+\sigma_{y}}{2} \pm R$
$\Uparrow$ Principal Normal Stresses

The directions are given by $\theta_{\mathrm{p} 1}$ and $\theta_{\mathrm{p} 2}$
Note that $2 \theta_{p 2}=2 \theta_{p 1}+\pi \Rightarrow \theta_{p 1} \perp \theta_{p 2}$
Also note that $\tau_{x^{\prime} y^{\prime}}=0$ on the planes which the principal normal stresses act.

## maximum Shear Stresses: $\Leftarrow$ sometimes called principal $\tau$

$\tau_{x^{\prime} y^{\prime}}=f(\theta) \Rightarrow$ The value of $\theta_{s}$ can be obtained by setting $\frac{\mathrm{d} \tau_{x^{\prime} y^{\prime}}}{\mathrm{d} \theta}=0$.
$\Rightarrow \tan 2 \theta_{s}=\frac{-\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)}{\tau_{x y}}$
[Fig. (6)]
There are 2 possible values for $\theta_{\mathrm{s}}$
$\Rightarrow \sin 2 \theta_{s 1}=\frac{\left(\sigma_{x}-\sigma_{y}\right) / 2}{R}$

$\cos 2 \theta_{s 1}=\frac{-\tau_{x y}}{R}$
$\sin 2 \theta_{s 2}=\frac{-\left(\sigma_{x}-\sigma_{y}\right) / 2}{R} \quad ; \quad \cos 2 \theta_{s 2}=\frac{\tau_{x y}}{R}$
$\Rightarrow \sqrt{\tau_{\max }^{\min }} \boldsymbol{=} \pm \sqrt{\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)^{2}+\tau_{x y}^{2}}= \pm \boldsymbol{R}$
$\Uparrow$ Maximum (Principal) Shear Stresses

$$
\begin{equation*}
\sigma_{x^{\prime}}=\sigma_{y^{\prime}}=\frac{\sigma_{x}+\sigma_{y}}{2} \tag{10}
\end{equation*}
$$

$\Uparrow$ Normal Stresses on the Planes of Maximum (Principal) Shear Stresses
Note that $2 \theta_{s 2}=2 \theta_{s 1}+\pi \quad \Rightarrow \quad \theta_{s 1} \perp \theta_{s 2}$
Also note that $\tan 2 \theta_{\mathrm{p}}$ is the negative reciprocal of $\tan 2 \theta_{\mathrm{s}}: \tan 2 \theta_{p}=\mathbf{- 1} / \tan 2 \theta_{s}$
$\Rightarrow 2 \theta_{s}=2 \theta_{p}+\pi 2 \quad \Rightarrow \quad \theta_{s}=\theta_{p}+45^{\circ}$
Thus, there is a $45^{\circ}$-angle between the planes of principal normal and maximum shear stresses.

## Example 1:

## Given:

The state of stress shown

## Req'd:

a) The principal stresses \& directions
b) $\sigma \& \tau$ associated with an element oriented $10^{\circ} \mathrm{cw}$ of the element shown.


Show the results on properly oriented elements.
Use the equations for the solution.
Solution:
a)

$$
\sigma_{\max }=\frac{\sigma_{x}+\sigma_{y}}{2} \pm \sqrt{\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)^{2}+\tau_{x y}^{2}}
$$

$$
=\frac{-6+2}{2} \pm \sqrt{\left(\frac{-6-2}{2}\right)^{2}+(-3)^{2}}=-2 \pm 5
$$

$\Rightarrow \quad \sigma_{\max }=3 \mathrm{ksi} \quad ; \quad \sigma_{\min }=-7 \mathrm{ksi}$


$$
\tan 2 \theta_{p}=\frac{2 \tau_{\mathrm{xy}}}{\sigma_{x}-\sigma_{y}}=\frac{2(-3)}{-6-2}=0.75 \Rightarrow 2 \theta_{\mathrm{p} 1}=36.87^{\circ}, 2 \theta_{\mathrm{p} 2}=216.87^{\circ}\left(-143.13^{\circ}\right)
$$

Maximum shear stresses

$$
\underset{\substack{\tau_{\min }}}{ }= \pm \mathrm{R} \quad \Rightarrow \tau_{\max }=5 \mathbf{k s i} \quad ; \tau_{\min }=-5 \mathbf{k s i}
$$

$$
\tan 2 \theta_{\mathrm{s}}=-1 / 0.75 \Rightarrow \boldsymbol{\theta}_{\mathbf{s} \mathbf{1}}=\mathbf{- 2 6 . 5 7 ^ { \mathbf { 0 } }} ; \boldsymbol{\theta}_{\mathbf{s} 2}=\mathbf{6 3 . 4 3}{ }^{\mathbf{0}} ; \tau(-26.57)=-5 \mathrm{ksi}=\tau_{\min } \leftrightarrow \theta_{\mathrm{s} 1}
$$

b) From. Eqs. [1] to [4],

## Note that $\sum \sigma_{i}=-4$

$$
\sigma_{x^{\prime}}\left(-10^{0}\right)=-4.73 \mathrm{ksi}
$$

$$
\tau_{x^{\prime} y^{\prime}}\left(-10^{\circ}\right)=\tau_{y^{\prime} x^{\prime}}\left(-10^{0}\right)=-4.19 \mathrm{ksi}
$$

$\sigma_{\mathbf{x}^{\prime}}=\sigma_{\mathbf{y}^{\prime}}=\frac{-6+2}{2}=-2 \mathrm{ksi}$

$$
\sigma_{\mathrm{y}^{\prime}}\left(-10^{0}\right)=0.73 \mathrm{ksi}
$$



## Mohr's Circle:

The general equation of the circle is

$$
\begin{equation*}
(x-a)^{2}+(y-b)^{2}=R^{2} \tag{12}
\end{equation*}
$$

By rearranging equation [1] and squaring both sides of equations [1] \& [2], and then adding, the following equation is obtained:

$$
\begin{equation*}
\left(\sigma_{x^{\prime}}-\frac{\sigma_{x}+\sigma_{y}}{2}\right)^{2}+\tau_{x y^{\prime}}^{2}=\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)^{2}+\tau_{x y}^{2} \tag{13}
\end{equation*}
$$

By comparing eq. [13] with eq. [12], it can be seen that

$$
\begin{aligned}
& x=\sigma_{x}^{\prime} \\
& a=\left(\sigma_{x}+\sigma_{y}\right) / 2=\sigma_{\text {average }} \\
& y=\tau_{x y^{\prime}} \\
& b=0 \\
& \boldsymbol{R}^{2}=\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)^{2}+\tau_{x y}^{2}
\end{aligned}
$$

Thus, by constructing a circle with the properties above, and by referring to Fig. (5), the values of $\sigma_{x^{\prime}}, \sigma_{y^{\prime}}$, and $\tau_{x^{\prime} y^{\prime}}$ with any $\boldsymbol{\theta}$ can be found; this includes the principal stresses and their directions.

This circle is called Mohr's Circle because Mohr brought the idea of such a circle. It has several applications, other than stresses. $\& \tau$ with any $\theta$ :
(1) Draw the $\sigma$ $\tau$ axes in the $x-y$ (i.e., horizontal-vertical) directions with"appropriate" scale. Note the direction of $+\tau$ (down).
(2) Put the points $x\left(\sigma_{x}, \tau_{x y}\right)$ and $y\left(\sigma_{y},-\tau_{x y}\right)$ on the figure.
(3) Connect the two points $x$ and $y$ by a straight line. The point of intersection of the line $x y$ and the $\sigma$-axis is the center of the circle $C$, and $c x \&$ cy are two radii of such a circle.
(4) Construct the circle with C as its center and Cx (or Cy) the radius.
(5) The points of intersection of the circle and the $\sigma$-axis are the principal normal stresses; the one to the right is the maximum, and the one to the left is the minimum.
(6) The radius of the circle is $\tau_{\max }$, and $\tau_{\min }=-R$.
(7) The angle measured from the $x$-axis to $\sigma_{\max }$ gives $2 \theta_{p 1(o r ~ p 2)}$ (i.e., double the angle), and the angle measured from $x$ to $\sigma_{\text {min }}$ is $2 \theta_{p 2(\text { or } p 1)}$.
(8) The angle measured from $x$ to $\tau_{\max }$ is $2 \theta_{s 1(\text { or } s 2)}$ and the angle from $x$ to $\tau_{\text {min }}$ is $2 \theta_{s 2(o r s 1)}$.
(9) The similar triangles show in Fig. (7) are used to calculate the required values (stresses and their directions).
(10) To calculate the stresses on planes oriented $\theta^{\circ}$ from the $x$-axis on the real plane, go $\underline{2} \theta$ from the $\boldsymbol{x}$-axis on the imaginary plane (Mohr's circle), and then draw a straight line passing through C. Use the triangles shown in Fig. (7) to determine the stress values.

Remember:

## * Start from x, Double the angle

* Some books use $+\tau$ as up (not down as done here). If it is so, then all angles will be in the opposite directions. You can go in the same direction (not the opposite) if one of the following is done:
(1) Reverse the "sign convention"
(2) Plot $x\left(\sigma_{x},-\tau_{x y}\right), y\left(\sigma_{y}, \tau_{x y}\right)$.
(3) Plot $+\tau$ axis down (as done here).



## Example 2:

Rework Example 1 using Mohr's circle.

## Solution:

a) The steps given above will be followed.


$$
\mathrm{OC}=\frac{\mathrm{OE}+\mathrm{OD}}{2}=\frac{2-6}{2}=-2
$$

Using the triangle CxD or $\mathrm{CyE}, \mathrm{R}$ and $2 \theta_{\mathrm{p}}$ can be calculated. $\Rightarrow$ $\tan 2 \theta_{p I}=\frac{x D}{C D}=\frac{|x D|}{|O D|-|O C|}$

$$
=\frac{3}{6-2}=0.75
$$


$\Rightarrow 2 \theta_{\mathrm{p} 1}=36.87^{\circ} \Rightarrow$
$\left.\Rightarrow \theta_{\mathrm{p} 1}=18.43^{\circ} \quad \operatorname{ccw}()\right)$ measured from the x -axis to the axis of $\sigma_{\text {min }}$

* Remember: Double the angle measured from $x$
$\Rightarrow 2 \theta_{\mathrm{p} 2}=180^{\circ}-2 \theta_{\mathrm{p} 1}$
$\Rightarrow \underline{\underline{\theta}}_{\mathrm{p} 2}=71.57^{\circ} \mathrm{CW}$ ()) measured from x to $\sigma_{\underline{\max }}$

$$
\begin{aligned}
& \mathbf{R}=\mathrm{Cx}=\mathrm{Cy}=|\mathrm{xD}| / \sin 2 \theta_{\mathrm{p} 1}=3 / \sin 36.87^{\circ}=\mathbf{5} \\
& \sigma_{\max }=\mathrm{OB}=\mathrm{CB}-|\mathrm{OC}|=\mathrm{R}-|\mathrm{OC}|=5-2 \Rightarrow \underline{\underline{\sigma_{\max }}=\mathbf{3} \mathbf{~ k s i}} \\
& \sigma_{\min }=-|\mathrm{OA}|=-(|\mathrm{OC}|+|\mathrm{CA}|)=-(|\mathrm{OC}|+\mathrm{R})=-(2+5) \Rightarrow \underline{\underline{\sigma_{\min }}=\mathbf{- 7} \mathbf{~ k s i}}
\end{aligned}
$$



Maximum shear stresses

Take care of the signs by inspection.

$$
\tau_{\max }= \pm \mathrm{R} \quad \Rightarrow \underline{\tau_{\max }}=\mathbf{5 k s i} \quad ; \quad \underline{\tau}_{\min }=-5 \mathrm{ksi}
$$

$$
\begin{aligned}
& 2 \theta_{\mathrm{s} 1}=90^{\circ}-2 \theta_{\mathrm{p} 1}=90-36.87^{\circ} \Rightarrow \underline{\left.\theta_{\mathrm{s} 1}=\mathbf{2 6 . 5 7 ^ { \circ }} \mathbf{\mathrm { cw }}()\right) \text { measured from } \mathrm{x} \text { to } \tau_{\underline{\min }}} \\
& 2 \theta_{\mathrm{s} 2}=90^{\circ}+2 \theta_{\mathrm{p} 1}=90+36.87^{\circ} \Rightarrow \underline{\underline{\theta_{s 2}}} \underline{\left.\underline{63.43^{\circ}} \mathbf{c c w}()\right) \text { measured from } \mathrm{x} \text { to } \tau_{\max }}
\end{aligned}
$$

b) $\sigma_{\mathrm{x}^{\prime}}\left(-10^{\circ}\right)=\mathrm{OF}=-(|\mathrm{OC}|+|\mathrm{CF}|)=-\left[|\mathrm{OC}|+\mathrm{R} \cos \left(2 \theta_{\mathrm{p} 1}+20^{\circ}\right)\right]=-\left[2+5 \cos \left(56.87^{\circ}\right)\right] \Rightarrow$

$$
\underline{\sigma}_{\underline{x}^{\prime}}(-10)=-4.73 \mathrm{ksi}
$$



Example 3:
Given:
The beam shown


## Req'd.:

Qualitatively, sketch the state of stress and Mohr's circle for each of the points A to G.

## Solution:

Note that $\sigma_{y}$ is always assumed zero (ignored) in beams.





## Example 4:

## Given:

The beam shown


## Req'd.:

The principal normal and shear stresses and their directions at the points) of maximum stresses.

## Solon.:

$\mathrm{M}_{\max }=450$ int $\underset{\mathrm{c}}{\mathrm{T}} \downarrow$ @ D
This M will give $\sigma_{\max }$ (both $\mathrm{T} \& \mathrm{C}$ )
$\mathrm{V}_{\text {max }}=45 \mathrm{k}$ @ D also
$\sigma=\sigma_{N}+\sigma_{M}= \pm \frac{N}{A} \pm \frac{M y}{I}$
$\sigma_{\text {top }}=\frac{-48}{24}+\frac{450(3)}{136}=-2+9.926$


$$
=7.926 \mathrm{ksi} \quad(\mathrm{~T})
$$

$\sigma_{\text {bottom }}=-\frac{48}{24}-\frac{450(5)}{136}=-2-16.54$

$$
=18.54 \mathrm{ksi} \quad(\mathrm{C})
$$

$\sigma_{\text {C.A. }}=\frac{-48}{24}+0=2 \mathrm{ksi}$
$\tau_{\text {top }}=\tau_{\text {bottom }}=0$
$\tau_{\text {C.A. }}=\tau_{\text {max }}=\frac{\mathrm{VQ}}{\mathrm{Ib}}=45(5 \times 2 \times 2.5) / 136(2)=4.136 \mathbf{~ k s i}$

## $\boldsymbol{\sigma}$ 's above are all $\sigma_{x}$.

$\sigma_{y}=0$ at all points (always the case in beam theory)

We need to calculate the principal stresses at 3 points (top, bottom, and N.A. of section D ). $\Rightarrow$ Choose the maximum normal ( $\mathrm{T} \& \mathrm{C}$ ) and shear stresses.

1) Top of D: (Mohr's circle is used for the max/min stress values below, not shown)
$\sigma_{\mathrm{x}}=7.926 \mathrm{ksi} \quad, \sigma_{\mathrm{y}}=0 \quad, \quad \tau_{\mathrm{xy}}=0$
$\Rightarrow \sigma_{\max }=7.926 \mathrm{ksi}, \quad \sigma_{\min }=0, \quad \tau_{\max }=3.963 \mathrm{ksi}, \quad \tau_{\min }=-3.963 \mathrm{ksi}$
2) Bottom of $D: \sigma_{x}=-18.54 \mathrm{ksi}, \quad \sigma_{y}=0, \quad \tau_{x y}=0$
$\Rightarrow \sigma_{\max }=0 \quad, \quad \sigma_{\min }=-18.54 \mathrm{ksi} \quad, \quad \tau_{\max }=9.27 \mathrm{ksi} \quad, \quad \tau_{\min }=-9.27 \mathrm{ksi}$
3) Centroidal Axis: $\sigma_{x}=-2 \mathrm{ksi}, \quad \sigma_{y}=0, \quad \tau_{\mathrm{xy}}=4.136 \mathrm{ksi}$

$$
\Rightarrow \sigma_{\max }=3.2 \mathrm{ksi} \quad, \quad \sigma_{\min }=-5.2 \mathrm{ksi} \quad, \quad \tau_{\max }=4.2 \mathrm{ksi} \quad, \quad \tau_{\min }=-4.2 \mathrm{ksi}
$$

$$
\begin{array}{ll}
\sigma_{\max }^{T}=7.926 k s i & @ \mathrm{D} \quad \text { (Top) } \\
\sigma_{\max }^{C}=18.54 \mathrm{ksi} & @ \mathrm{D}(\text { Bottom }) \\
\tau_{\max }=9.27 \mathrm{ksi} & \text { @ } \mathrm{D} \text { (Bottom) }
\end{array}
$$

Important note: When $\tau_{x y}$ is zero at a point, then $\sigma_{x}$ and $\sigma_{y}$ are themselves principal stresses at that particular point (as seen above).

