Transformation of Stress

Theory & Examples

- * <u>**Triaxial</u>** states of stress are shown in Fig. (1). Only positive stresses on positive faces are shown</u>
- * <u>**Biaxial</u>** states of stress (Plane Stress):</u>

When all stresses act in the same plane.

 \Rightarrow Work in 2-D as shown in Fig. (2) in the

<u>*x-y*</u> directions.



It is possible to have the orientation in different directions



20

From Fig. (4), the sum of **forces** in the x' and y'

directions must be zero.

* Note that <u>forces not stresses</u> are added.



Recall that

 $\cos^{2} \theta = (1 + \cos 2\theta)/2$ $\sin^{2} \theta = (1 - \cos 2\theta)/2$ $\sin \theta \cos \theta = \sin 2\theta/2$ $\cos^{2} \theta - \sin^{2} \theta = \cos 2\theta$

 \Rightarrow

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta \qquad [1]$$

$$\tau_{x'y'} = -\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$
^[2]

Similarly,

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$
^[3]

$$\tau_{y'x'} = \tau_{x'y'}$$
^[4]

* Note that $\sigma_x + \sigma_y = \sigma_{x'} + \sigma_{y'} = I$ (Invariant with <u>any</u> θ) \Leftrightarrow Use it to <u>check</u>.

When $\sigma \& \tau$ on <u>any</u> two orthogonal faces are known, the stress components on <u>all</u> (any) faces (plane stress) can be calculated.





Principal Normal Stresses:

$$\sigma_{x'} = f(\theta) \Rightarrow \text{ to get } \sigma_{x'max} , \text{ set } \frac{d\sigma_{x'}}{d\theta} = 0 \Rightarrow \text{ find } \theta_p \Rightarrow \sigma_{x'max}$$

$$\frac{d\sigma_{x'}}{d\theta} = -(\sigma_x - \sigma_y) \sin 2\theta_p + 2\tau_{xy} \cos 2\theta_p = 0$$

$$\text{Dividing by } \cos 2\theta, \qquad \tan 2\theta_p = \frac{2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{\tau_{xy}}{(\sigma_x - \sigma_y)/2} \qquad [5]$$

From the equation above, Fig. (5) shown can be constructed.



1 Principal Normal Stresses

The directions are given by θ_{p1} and θ_{p2}

Note that $2\theta_{p2} = 2\theta_{p1} + \pi \implies \theta_{p1} \perp \theta_{p2}$

Also note that $\tau_{x'y'} = 0$ on the <u>planes</u> which the <u>principal normal stresses</u> act.

<u>maximum Shear Stresses</u>: \leftarrow sometimes called <u>principal</u> τ

[8]

On-oy

2052

Fig. 6

xy

205

 γ_{xy}

 $\tau_{x'y'} = f(\theta) \implies \text{The value of } \theta_s \text{ can be obtained by setting } \frac{d\tau_{x'y'}}{d\theta} = 0.$

$$\Rightarrow \tan 2\theta_s = \frac{-\left(\frac{\sigma_x - \sigma_y}{2}\right)}{\tau_{xy}}$$

There are 2 possible values for θ_s

$$\Rightarrow \sin 2\theta_{s1} = \frac{(\sigma_x - \sigma_y)/2}{R}$$

 $\cos 2\theta_{s1} = \frac{-\tau_{xy}}{R}$

$$\sin 2\theta_{s2} = \frac{-(\sigma_x - \sigma_y)/2}{R} \qquad ; \qquad \cos 2\theta_{s2} = \frac{\tau_{xy}}{R}$$
$$\Rightarrow \boxed{\tau_{\max}}_{\min} = \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} = \pm R \qquad [9]$$

Maximum (Principal) Shear Stresses

$$\sigma_{x'} = \sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} \qquad [10]$$

Normal Stresses on the Planes of Maximum (Principal) Shear Stresses

Note that $2\theta_{s2} = 2\theta_{s1} + \pi \implies \theta_{s1} \perp \theta_{s2}$

Also note that $\tan 2\theta_p$ is the negative reciprocal of $\tan 2\theta_s$: $\tan 2\theta_p = -1/\tan 2\theta_s$

$$\Rightarrow 2\theta_s = 2\theta_p + \pi/2 \quad \Rightarrow \quad \theta_s = \theta_p + 45^\circ$$
^[11]

Thus, there is a 45°-angle between the planes of principal normal and maximum shear stresses.

Example 1:

Given:

The state of stress shown

Req'd:

- a) The principal stresses & directions
- b) $\sigma \& \tau$ associated with an element oriented 10° cw of the element shown.

Show the results on properly oriented elements.

Use the equations for the solution.

Solution:

$$\sigma_{\max_{min}} = \frac{\sigma_{x} + \sigma_{y}}{2} \pm \sqrt{\left(\frac{\sigma_{x} - \sigma_{y}}{2}\right)^{2} + \tau_{xy}^{2}}$$
$$= \frac{-6 + 2}{2} \pm \sqrt{\left(\frac{-6 - 2}{2}\right)^{2} + (-3)^{2}} = -2 \pm \sqrt{\left(\frac{-6 - 2}{2}\right)^{2} + (-3)^{2}}$$

$$\Rightarrow \sigma_{\max} = 3 \text{ ksi} \quad ; \quad \sigma_{\min} = -7 \text{ ksi}$$





5

To see θ_{p1} corresponds to σ_{max} or σ_{min} , substitute θ in Eq. [1] 18.4349 by Θ_{p1}

$$\Rightarrow \sigma_{x'} = \frac{-6+2}{2} + \frac{-6-2}{2} \cos 36.87^{\circ} - 3 \sin 36.57^{\circ} = -7 \, \text{ksi}$$

 $\Rightarrow \, \theta_{p1}$ is the direction of σ_{min} as shown.

$$\tau_{\max} = \pm R \quad \Rightarrow \boxed{\tau_{\max} = 5 \text{ ksi}} \quad ; \quad \tau_{\min} = -5 \text{ ksi}} \qquad \sigma_{x'} = \sigma_{y'} = \frac{-6+2}{2} = -2 \text{ ksi}$$

$$\tan 2\theta_s = -1/0.75 \Rightarrow \boxed{\theta_{s1} = -26.57^\circ} \quad ; \quad \theta_{s2} = 63.43^\circ} \quad ; \quad \tau(-26.57) = -5 \text{ ksi} = \tau_{\min} \leftrightarrow \theta_{s1}$$
b) From. Eqs. [1] to [4],
$$\boxed{\sigma_{x'} (-10^\circ) = -4.73 \text{ ksi}} \quad ; \quad \sigma_{y'} (-10^\circ) = 0.73 \text{ ksi}}$$
Note that $\sum \sigma_i = -4 \qquad (always)$

$$\boxed{\tau_{x'y'} (-10^\circ) = \tau_{y'x'} (-10^\circ) = -4.19 \text{ ksi} (\pm)}$$
Part (b)
Part (b)

X

26.57

/ 2 ksi 5 ksi

Mohr's Circle:

The general equation of the circle is

$$(x-a)^{2} + (y-b)^{2} = R^{2}$$
[12]

By rearranging equation [1] and squaring both sides of equations [1] & [2], and then adding, the following equation is obtained:

$$\left(\sigma_{x'} - \frac{\sigma_x + \sigma_y}{2}\right)^2 + \tau_{x'y'}^2 = \left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2 \qquad [13]$$

By comparing eq. [13] with eq. [12], it can be seen that

$$x = \sigma_{x'}$$

$$a = (\sigma_x + \sigma_y)/2 = \sigma_{average}$$

$$y = \tau_{x'y'}$$

$$b = 0$$

$$R^2 = \left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2$$

Thus, by constructing a circle with the properties above, and by referring to Fig. (5), the values of $\sigma_{x'}$, $\sigma_{y'}$, and $\tau_{x'y'}$ with any θ can be found; this includes the principal stresses and their directions. This circle is called Mohr's Circle because Mohr brought the idea of such a circle. It has several applications, other than stresses.

<u>Steps for constructing Mohr's circle and determining the principal stresses & directions and σ & τ with any θ :</u>

- (1) Draw the σ - τ axes in the x-y (i.e., horizontal-vertical) directions with "appropriate" scale. Note the direction of $+\tau$ (down).
- (2) Put the points $x(\sigma_x, \tau_{xy})$ and $y(\sigma_y, -\tau_{xy})$ on the figure.
- (3) Connect the two points x and y by a straight line. The point of intersection of the line xy and the σ-axis is the center of the circle C, and cx & cy are two radii of such a circle.
- (4) Construct the circle with C as its center and Cx (or Cy) the radius.
- (5) The points of intersection of the circle and the σ -axis are the principal normal stresses; the one to the right is the maximum, and the one to the left is the minimum.
- (6) The radius of the circle is τ_{max} , and $\tau_{min} = -R$.
- (7) The angle measured from the x-axis to σ_{max} gives $2\theta_{p1 (or p2)}$ (i.e., double the angle), and the angle measured from x to σ_{min} is $2\theta_{p2 (or p1)}$.
- (8) The angle measured from x to τ_{max} is $2\theta_{s1 (or s2)}$ and the angle from x to τ_{min} is $2\theta_{s2 (or s1)}$.
- (9) The similar triangles show in Fig. (7) are used to calculate the required values (stresses and their directions).
- (10) To calculate the stresses on planes oriented θ° from the x-axis on the real plane, go $\underline{2\theta}$ from the <u>x-axis</u> on the imaginary plane (Mohr's circle), and then draw a straight line passing through C. Use the triangles shown in Fig. (7) to determine the stress values.

Remember:

* Start from x, Double the angle

* Some books use $+\tau$ as up (not down as done here). If it is so, then all angles will be in the <u>opposite</u> directions. You can go in the same direction (not the opposite) if one of the following is done:

(1) Reverse the "sign convention" for shear.



- (2) Plot $x(\sigma_x, -\tau_{xy}), y(\sigma_y, \tau_{xy})$.
- (3) Plot + τ axis down (as done here).



Example 2:

Rework Example 1 using Mohr's circle.

Solution:

a) The steps given above will be followed.





$$OC = \frac{OE + OD}{2} = \frac{2 - 6}{2} = -2$$

Using the triangle CxD or CyE, R and $2\theta_p$ can be calculated. \Rightarrow

$$\tan 2\theta_{p1} = \frac{xD}{CD} = \frac{|xD|}{|OD| - |OC|}$$
$$= \frac{3}{6-2} = 0.75$$

 $\Rightarrow 2\theta_{p1} = 36.87^{\circ} \Rightarrow$

 $\Rightarrow \theta_{p1} = 18.43^{\circ}$ ccw () measured from the x-axis to the axis of σ_{min}





Example 3:

Given:

The beam shown A A Centroidol azis A Centroidol azis $D = (\sigma_z = 0)$ A MG

Req'd.:

Qualitatively, sketch the **state of stress** and **Mohr's circle** for each of the **points A to G**.

Solution:

Note that σ_{y} is always assumed zero (ignored) in beams.











Example 4:

Given:

The beam shown



Req'd.:

The principal normal and shear stresses and their directions at the point(s) of maximum stresses.

Sol'n.:



= **7.926 ksi** (**T**)



$\sigma_y = 0$ at all points (always the case in <u>beam</u> theory)

We need to calculate the principal stresses at 3 points (top, bottom, and N.A. of section D). \Rightarrow Choose the maximum normal (T & C) and shear stresses.

1) Top of D : (Mohr's circle is used for the max/min stress values below, not shown)

$$\begin{split} \sigma_x &= 7.926 \text{ ksi} \quad , \ \sigma_y &= 0 \quad , \qquad \tau_{xy} = 0 \\ \Rightarrow & \sigma_{max} &= 7.926 \text{ ksi} \quad , \qquad \sigma_{min} = 0 \quad , \qquad \tau_{max} = 3.963 \text{ ksi} \quad , \qquad \tau_{min} = -3.963 \text{ ksi} \end{split}$$

2) Bottom of D: $\sigma_x = -18.54 \text{ ksi}$, $\sigma_y = 0$, $\tau_{xy} = 0$

 $\Rightarrow \ \sigma_{max} = 0 \quad , \quad \sigma_{min} = -18.54 \ ksi \quad , \quad \tau_{max} = 9.27 \ ksi \quad , \ \tau_{min} = -9.27 \ ksi$

3) Centroidal Axis: $\sigma_x = -2 \text{ ksi}$, $\sigma_y = 0$, $\tau_{xy} = 4.136 \text{ ksi}$

 $\Rightarrow \sigma_{max} = 3.2 \text{ ksi}$, $\sigma_{min} = -5.2 \text{ ksi}$, $\tau_{max} = 4.2 \text{ ksi}$, $\tau_{min} = -4.2 \text{ ksi}$

$\sigma_{max}^{T} = 7.926 \ ksi$	@ D (Top)
$\sigma_{max}^{C} = 18.54 \ ksi$	@ D (Bottom)
$\tau_{max} = 9.27 ksi$	@ D (Bottom)

* <u>Important note</u>: When τ_{xy} is zero at a point, then σ_x and σ_y are themselves principal stresses at that particular point (as seen above).