

#17

Examples (Bending Stresses)

Example 1:

Given:

The cross section & BMD shown for a beam

Req. d.:

The values & locations of the maximum positive and negative normal stresses

Soln.:

$$\textcircled{1} \text{ Find the centroid: } \bar{y} = \frac{\sum_{i=1}^n A_i \bar{y}_i}{\sum_{i=1}^n A_i}$$

$$\bar{y} = \frac{12(3) + 12(7)}{12 + 12} = 5 \text{ in}$$

$$\textcircled{2} \text{ Calculate } I: I = \sum_{i=1}^n (I_i + A_i d_i^2)$$

$$I = \left[\frac{1}{12} (2) 6^3 + 12 (2)^2 \right] + \left[\frac{1}{12} (6) 2^3 + 12 (2)^2 \right] = 136 \text{ in}^4$$

$$\textcircled{3} \text{ Determine max. pos. \& neg. } M: M_{\max}^+ = 408 \text{ in}\cdot\text{k}, M_{\max}^- = 544 \text{ in}\cdot\text{k}$$

$$\textcircled{4} \text{ Locate max } y^+ \& y^-: y_{\max}^+ = 3 \text{'' (top)}; y_{\max}^- = 5 \text{ in (bottom)}$$

$$\textcircled{5} \text{ Calculate max negative \& positive stresses}$$

$$\sigma = - \frac{My}{I}$$

$$\sigma_{\max C}^T = - \frac{408(-5)}{136} = 15 \text{ ksi (T) at the bottom of point C}$$

$$\sigma_{\max B}^T = - \frac{544(3)}{136} = 12 \text{ ksi (T) at the top of point B}$$

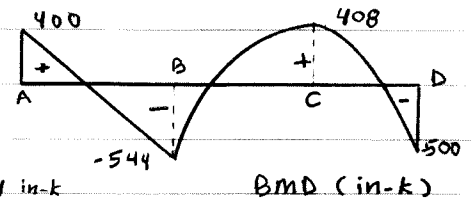
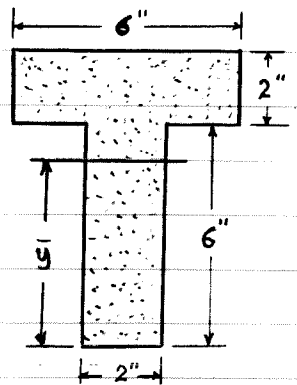
$$15 > 12 \Rightarrow \sigma_{\max}^T = 15 \text{ ksi (T) at the bottom of point C}$$

$$\sigma_{\max B}^C = - \frac{544(-5)}{136} = -20 \text{ ksi (C) at the bottom of point B}$$

Clearly, the other option ($M = +408$ and $y = +3$) does not control (i.e., it will not give a bigger value for σ) because both M and y are smaller.

$$\Rightarrow \sigma_{\max}^C = 20 \text{ ksi (C) at the bottom of point B}$$

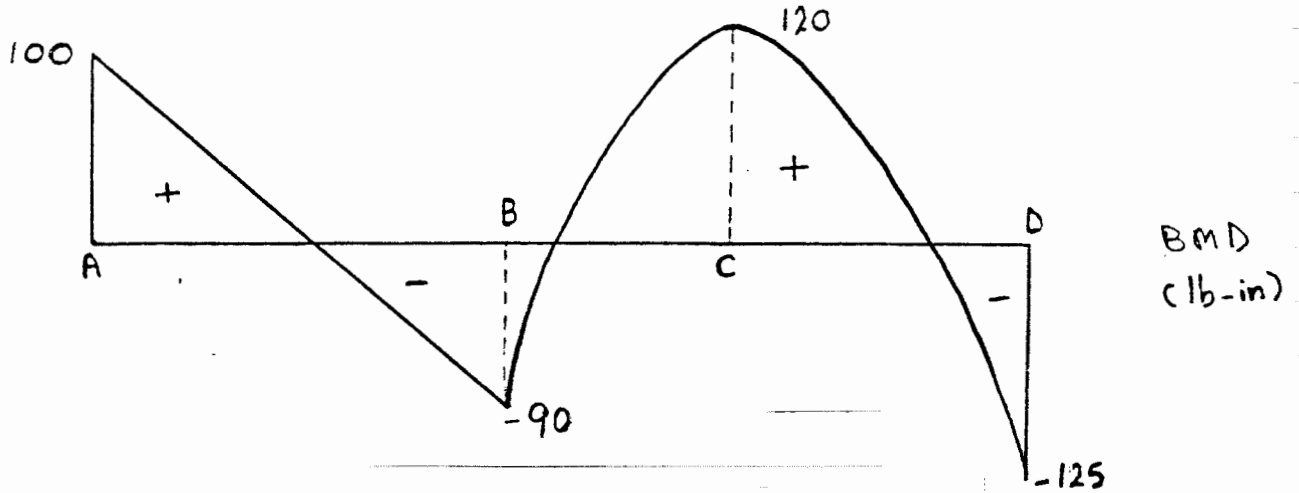
Important note: You need to check two points, one is the point of max. pos. M and the other is point of max. neg. M . For both points check the top and bottom of the cross section for max. ten. & comp. σ .



②

Example 2 :

The bending moment diagram (BMD) and the cross section of a beam are shown below. Determine the values and locations of the maximum positive and negative normal stresses.



Soln. :

The steps in Example 1 will be followed

$$\bar{y} = 3.633 \text{ in}$$

$$I = \frac{10(1)^3}{12} + 10(1)(7 - 3.633 - 0.5)^2 + 2 \left[\frac{1(4)^3}{12} + 1(4)(4 - 3.633)^2 \right] + \frac{6(2)^3}{12} + 6(2)(1 - 3.633)^2 = 182 \text{ in}^4$$

$$M_{max}^+ = 120 \text{ in-lb @ C}$$

$$M_{max}^- = -125 \text{ in-lb @ D}$$

$$\sigma = -My / I$$

Point C :

$$\sigma_{max}^T = \frac{-120(-3.633)}{182} = 2.40 \text{ psi (T) at bottom}$$

$$\sigma_{max}^C = -\frac{120(3.367)}{182} = 2.22 \text{ psi (C) at top (note - sign)}$$

Point D :

$$\sigma_{max}^T = -\frac{-125(3.367)}{182} = 2.31 \text{ psi (T) at top}$$

$$\sigma_{max}^C = -\frac{-125(-3.633)}{182} = 2.50 \text{ psi (C) at bottom (note - sign)}$$

$$\Rightarrow \sigma_{max}^T = 2.40 \text{ psi @ bottom of C} ; \sigma_{max}^C = 2.50 \text{ psi @ bottom of D}$$

③

Example 3:

Given:

The beam with the section shown

$\sigma_{max} = 30 \text{ MPa (C \& T)}$

Reqd.:

The minimum value of b (cross-sectional dimension)

Soln.:

$\sigma_{max} = \frac{M_{max} y_{max}}{I}$

$M_{max} = 20(4) = 80 \text{ kN.m (at the fixed end)}$

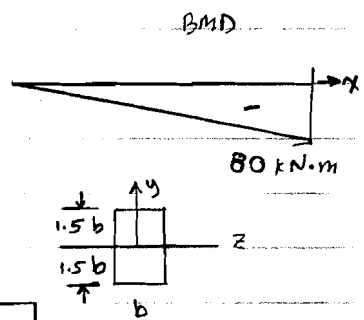
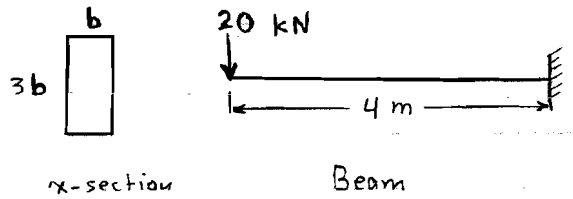
$y_{max} = 1.5 b$

$I = \frac{1}{12} b h^3 = \frac{1}{12} b (3b)^3 = \frac{9}{4} b^4$

$\Rightarrow 30(10^6) = 80(10^3)(1.5b) / \frac{9}{4} b^4$

$\Rightarrow b^3 = 0.0017778 \text{ m}^3 \Rightarrow$

$b = 0.121 \text{ m} = 121 \text{ mm}$
the section is 121 by 363 mm



Example 4:

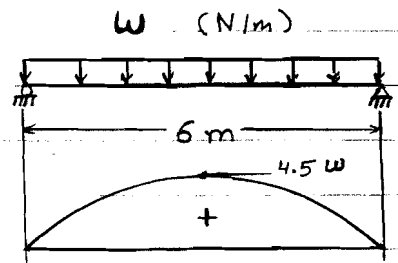
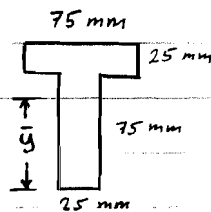
Given:

The beam & section shown

$\sigma_{max}^T = 40 \text{ MPa} ; \sigma_{max}^C = -25 \text{ MPa}$

Reqd.:

- a) The max. value of W
- b) The force in the flange @ point of M_{max}
- c) The moment of the force in the flange of (b) about N.A.
- d) Show that $M_{flange} = M_{BMD}$ & $N_{total} = 0$ (pure bending)



Soln.:

$\bar{y} = 62.5 \text{ mm} = 0.0625 \text{ m} ; I_{NA} = 3.32 (10)^6 \text{ m}^4 ; M_{max} = (\frac{WL}{2})(\frac{L}{2})(\frac{L}{2}) = 4.5 W$

a) $\sigma = -My/I \Rightarrow \sigma_{max}^T = 40 (10^6) = -4.5 W (-0.0625) / 3.32 (10)^6 \Rightarrow W_{max} = 472.2 \text{ N/m}$

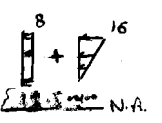
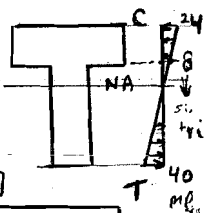
$\sigma_{max}^C = -25 (10^6) = -4.5 W (0.1 - 0.0625) / 3.32 (10)^6 \Rightarrow W_{max} = 491.9 \text{ N/m}$
 $\Rightarrow W_{max} = 472.2 \text{ N/m}$

b) $\sigma_{bottom} = +40 \text{ MPa} ; \sigma_{top} = -\frac{4.5(472.2)(0.0375)}{3.32(10)^6} = -24 \text{ MPa (C)}$

$F_{flange} = \sigma_{avg} A = \frac{24+8}{2} (0.075 \times 0.025) = -0.03 \text{ MN} = -30 \text{ kN (C)}$

c) $M_{NA} (F_{flange}) = 8(10^6) (0.075 \times 0.025) (0.0125 + 0.0125) + \frac{16(10^6)}{2} (0.075 \times 0.025) [0.0125 + \frac{2}{3}(0.025)]$

d) $\sum N = -0.03 - \frac{8}{2} (0.0125 \times 0.025) + \frac{40}{2} (0.0625 \times 0.025) = 0$
 $M_{flange} = 812.5 + 0.0125(10)^6 (\frac{2}{3} \times 0.0125) + 0.03125(10)^6 (\frac{2}{3} \times 0.0625) = 2125 \text{ N.m} = M_{BMD}$
 $\Rightarrow M_{flange} = 812.5 \text{ N.m}$



④

Example 5 :

Given :

The beam shown with steel channel section

$$\sigma_{max} = -18 \text{ ksi} \quad (\pm)$$

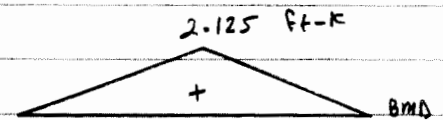
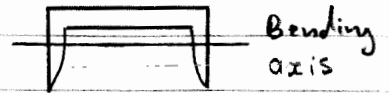
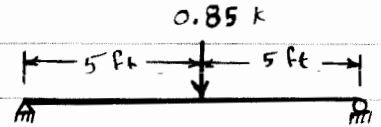
Req.d :

The section with least weight

Sol.n. :

From the BMD shown, $M_{max} = 2.125 \text{ ft-k}$.

The properties of several structural steel shapes are given in an appendix in the textbook.



$$\sigma_{max} = \frac{Mc}{I} \quad (\text{do not worry about the sign as we have } \pm 18 \text{ ksi})$$

$$18 = \frac{2.125 (12 \text{ in}) c}{I} \Rightarrow \frac{I}{c} = 1.42 \text{ in}^3 = S = \text{Section modulus}$$

[Explanation of designation in the appendix: examples, W 18 X 60, S 24 X 100, C 15 X 50. The first letter is for the shape (W = wide-flange, S = standard, C = channel, L = leg ... etc). The following number is the nominal depth in inches, and the last number is the weight in pound per foot (length). Thus, C 15 X 50 means channel section 15 in deep by 50 lb/in. The weight is the measurement of the cost.]

From the table in the book, we need to select the section with S closest to 1.42 in³ and with least weight. We may have more than one option. Note that in this example, bending is about the y axis, thus, we must use S_y.

options : ① C 12 X 20.7 (S_y = 1.73 in³ >> 1.42 ok)

② C 10 X 25 (S_y = 1.48 in³ > 1.42 ok)

Note that option ② has S_y closest to 1.42; however, option ① is cheaper (less weight as 20.7 < 25) and better (bigger S_y as 1.73 > 1.48) ⇒

Select C 12 X 20.7