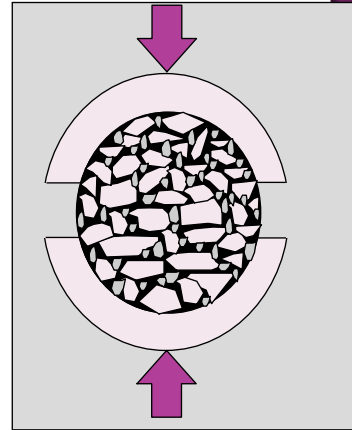


# MARSHALL MIX DESIGN



HAW

1

## REQUIREMENTS IN COMMON

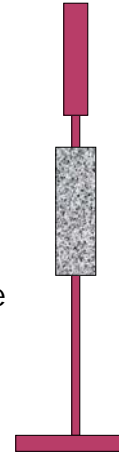
- ◉ Sufficient asphalt to ensure a durable pavement
- ◉ Sufficient stability under traffic loads
- ◉ Sufficient air voids
  - Upper limit to prevent excessive environmental damage
  - Lower limit to allow room for initial densification due to traffic
- ◉ Sufficient workability

HAW

2

## MARSHALL MIX DESIGN

- ◉ Developed by Bruce Marshall for the Mississippi Highway Department in the late 30's
- ◉ WES began to study it in 1943 for WWII
  - Evaluated compaction effort
    - No. of blows, foot design, etc.
    - Decided on 10 lb. Hammer, 50 blows/side
    - 4% voids after traffic
- ◉ Initial criteria were established and upgraded for increased tire pressures and loads



HAW

3

## AUTOMATIC MARSHALL HAMMER



HAW

4

## MARSHALL MIX DESIGN

- ◉ Select and test aggregate
- ◉ Select and test asphalt cement
  - Establish mixing and compaction temperatures
- ◉ Develop trial blends
  - Heat and mix asphalt cement and aggregates
  - Compact specimen (100 mm diameter)

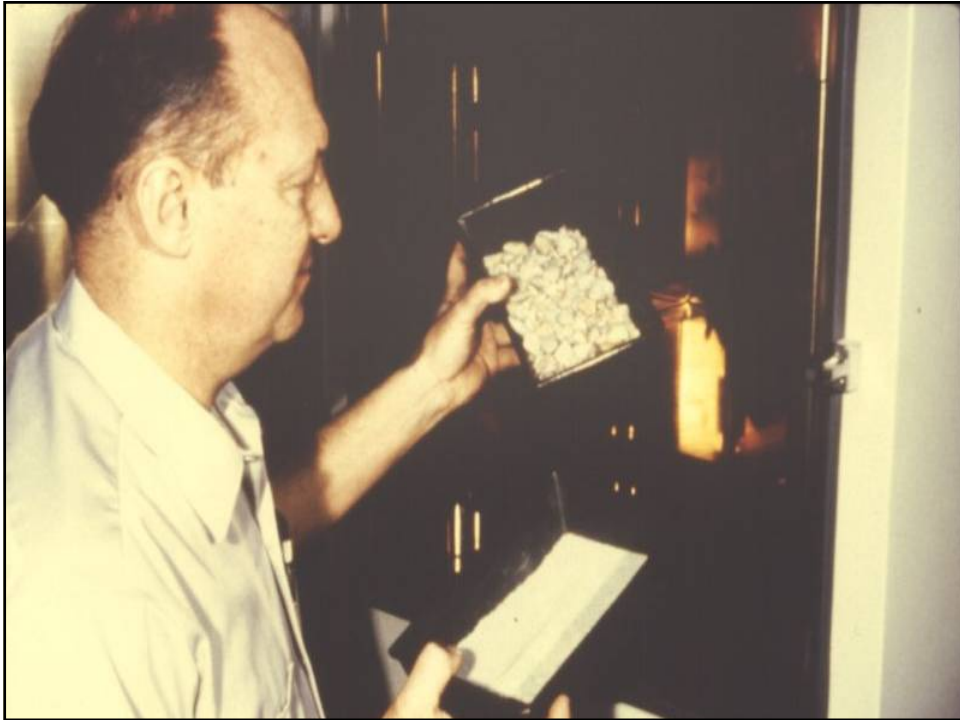




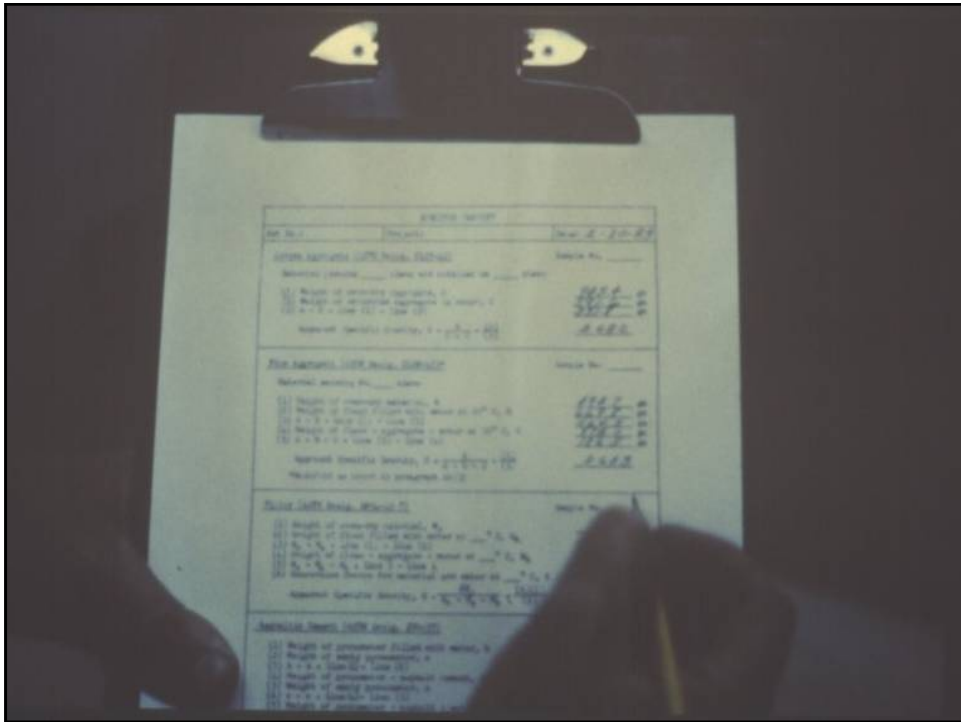
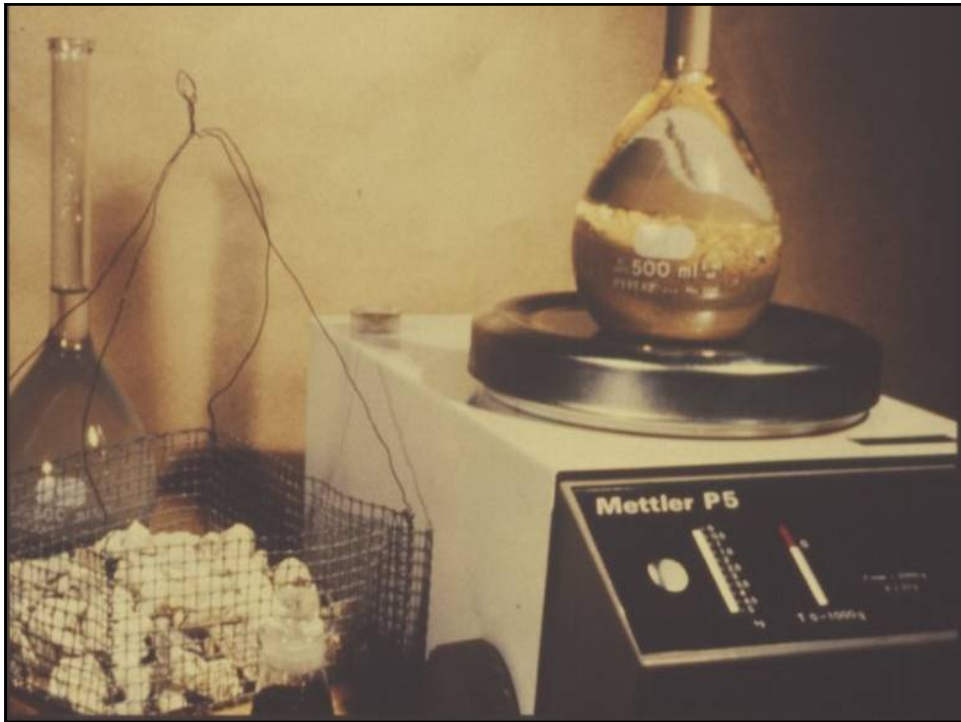
# Marshall Procedures

- Density & Voids Analysis
- Stability & Flow Test













-200	50.0	3.0	13.0
	50.0	5.0	10.0
Total	1000.0	5.0 (T)	5.0

(A) Weight original sample 1000 gm  
 (B) Weight after washed 950 gm  
 (C) Wash loss (A - B) 50 gm  
 (S) -200 from sieving 0 gm  
 (T) Total -200 C + S 50 gm

Use "T" to calculate percentages

Checked by: \_\_\_\_\_





SIZE	PERCENT PASSING	PERCENT PASSING	PERCENT PASSING
1 in	100		
3/4 in	80		
3/8 in	65		
3/16 in	35		
No. 4	0		
No. 8		100	
No. 30		85	
No. 50		60	
No. 100		40	100
No. 200		15	98
		5	95
			80



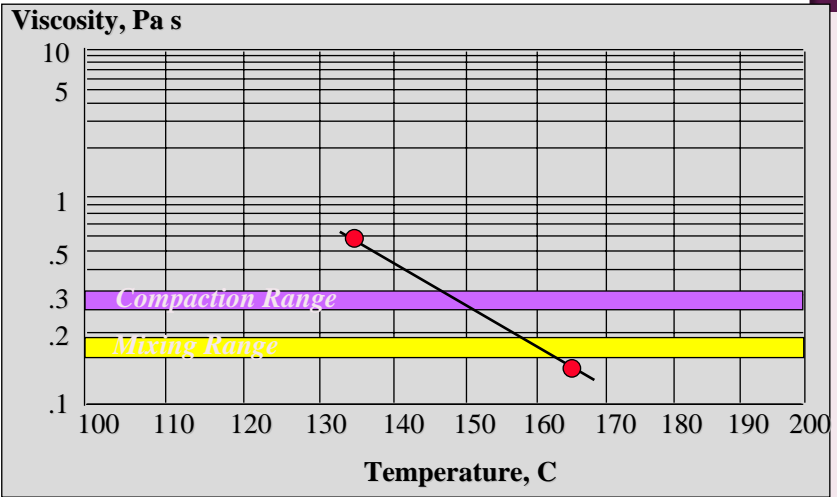
## **Recommended Aggregate Fractions**

**1 in – 3/4 in**  
**3/4 in – 3/8 in**  
**3/8 in – No.4**  
**No.4 – No.8**  
**Passing No.8**





# MIXING/COMPACTION TEMPS









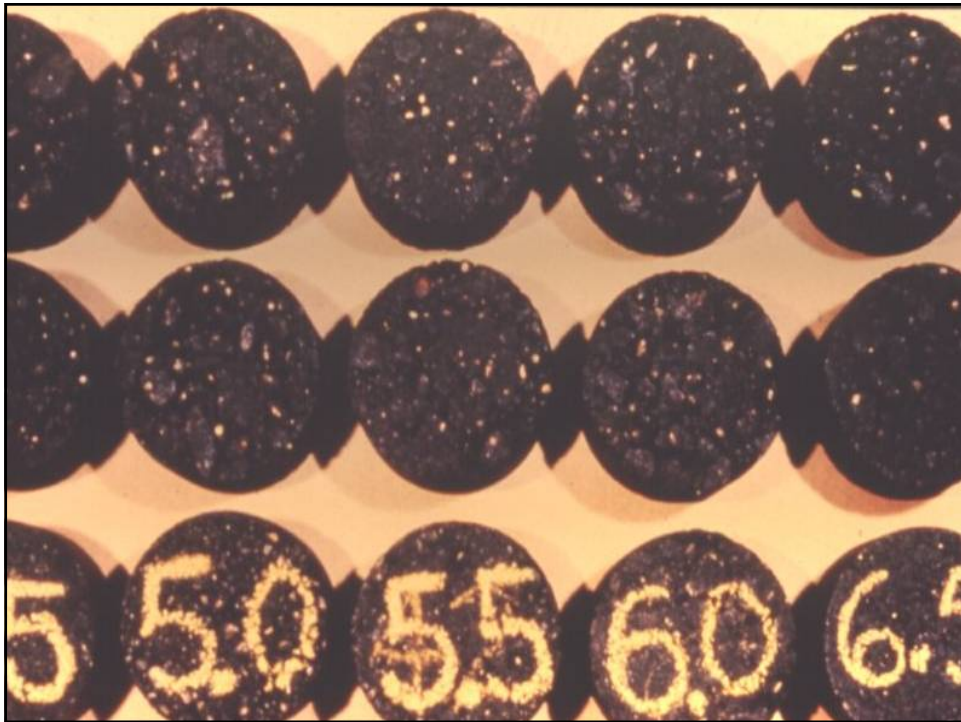


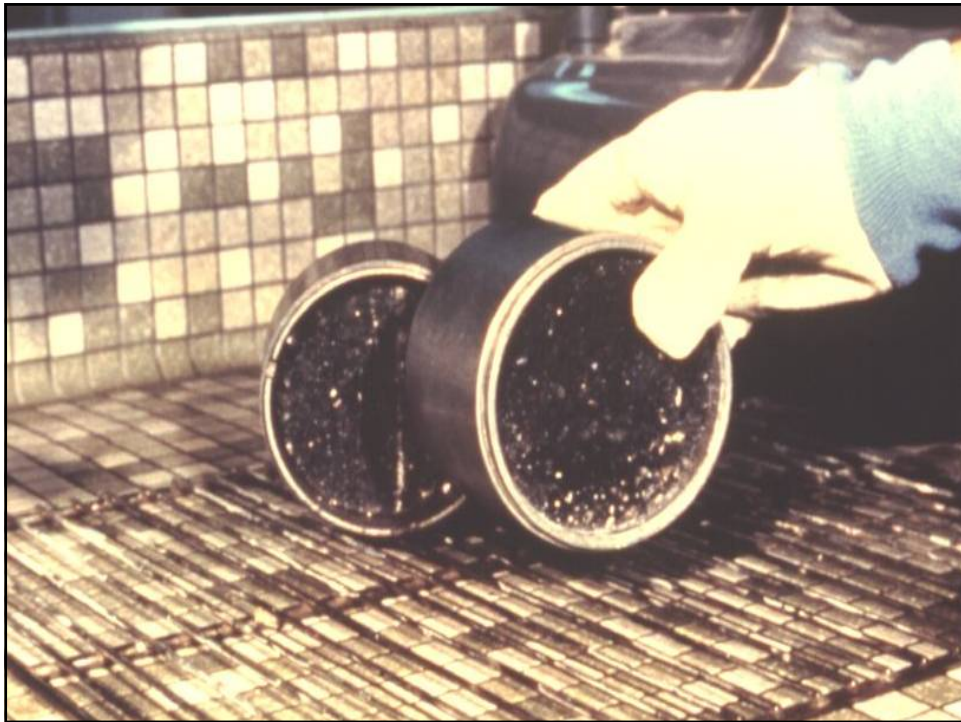






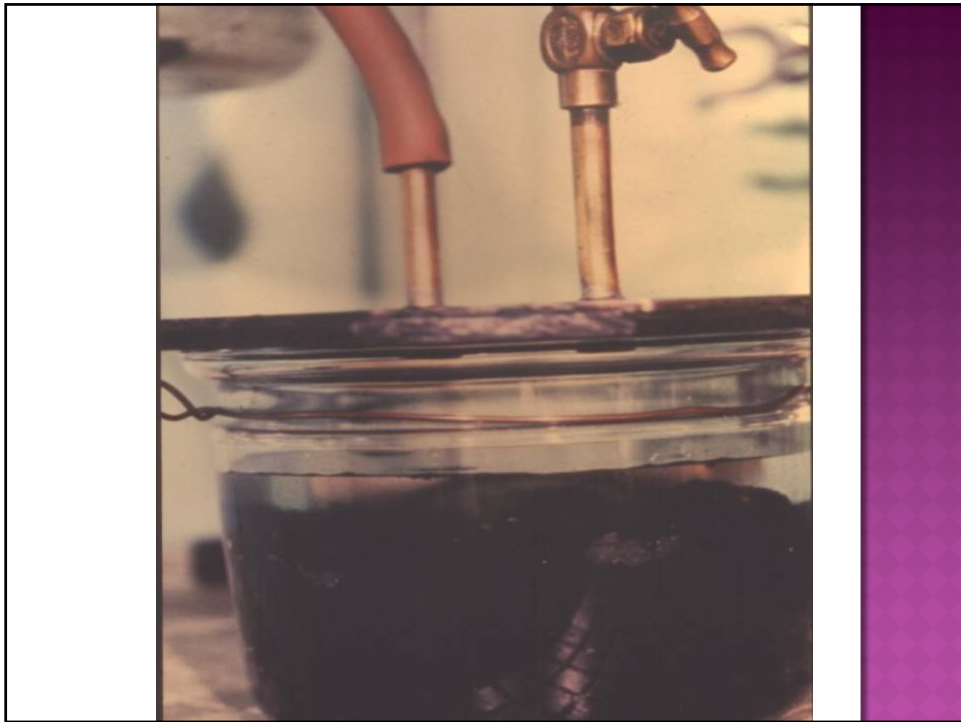


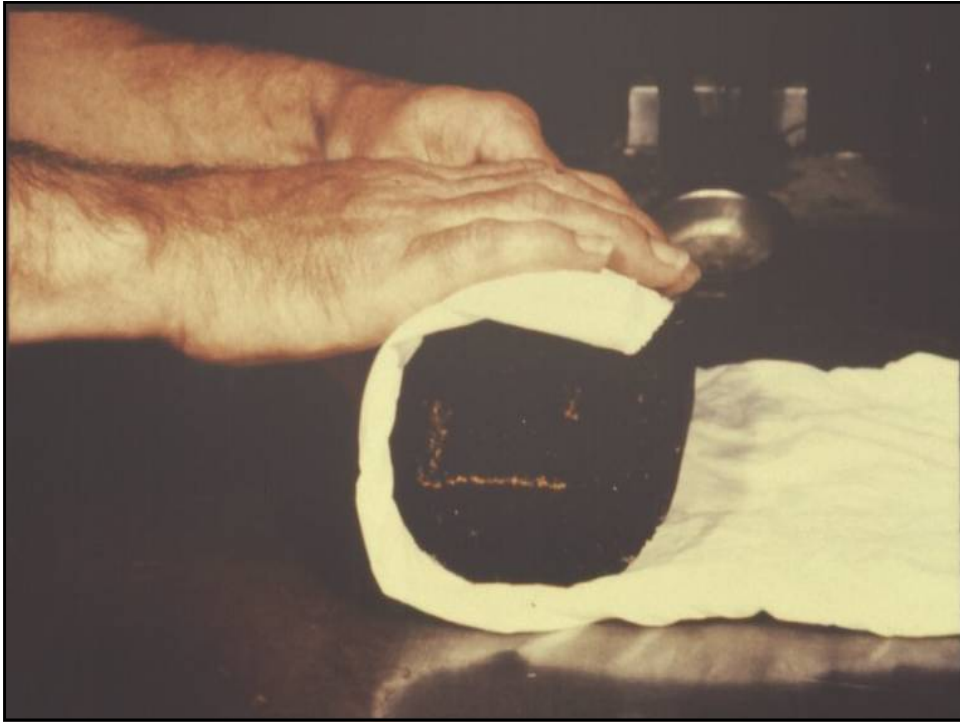




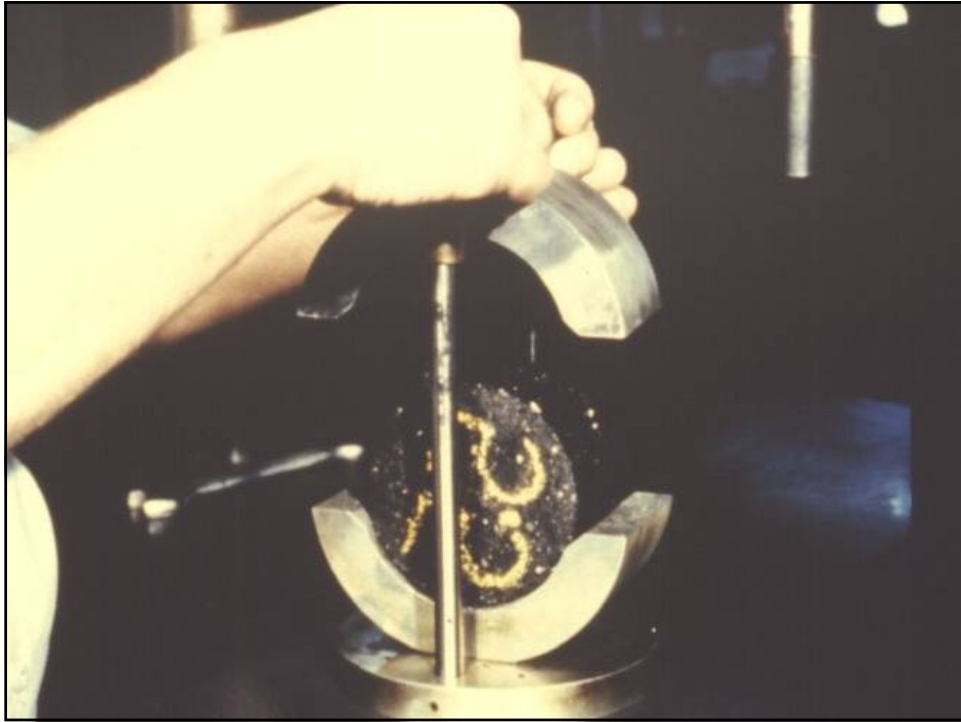
## MARSHALL MIX DESIGN TESTS

- ◉ Heights
  - Used to correct stability measurements
- ◉ Bulk specific gravity of compacted sample
- ◉ Maximum specific gravity of loose mix
- ◉ Stability and flow





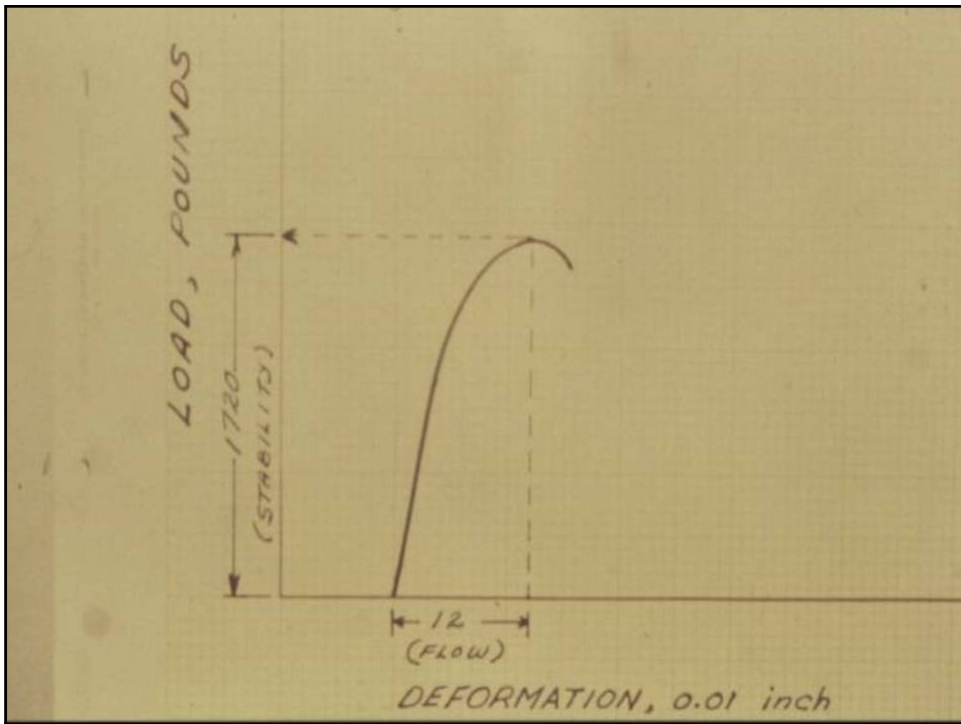
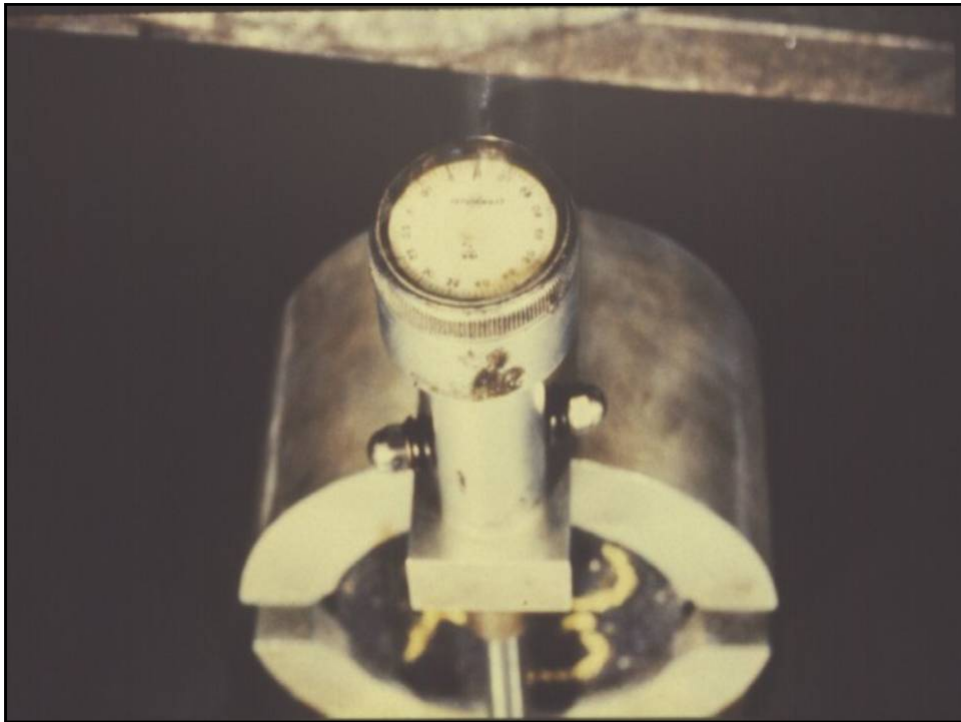


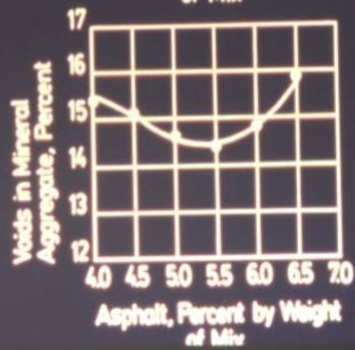
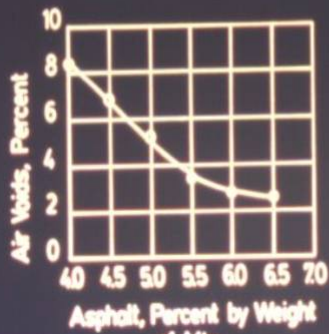
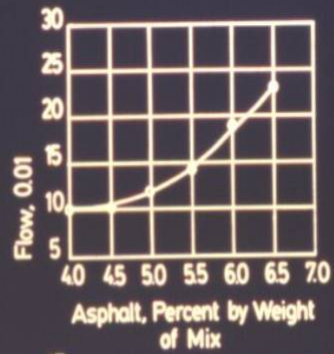
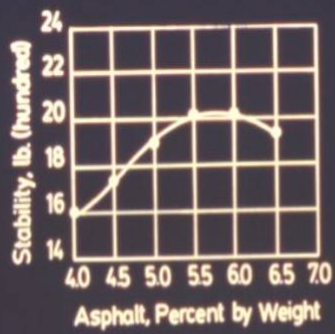
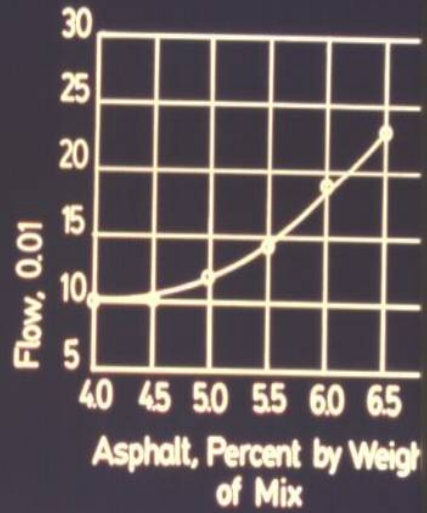
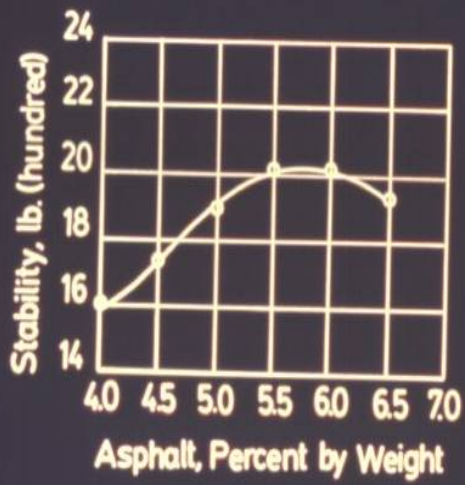


## MARSHALL STABILITY AND FLOW





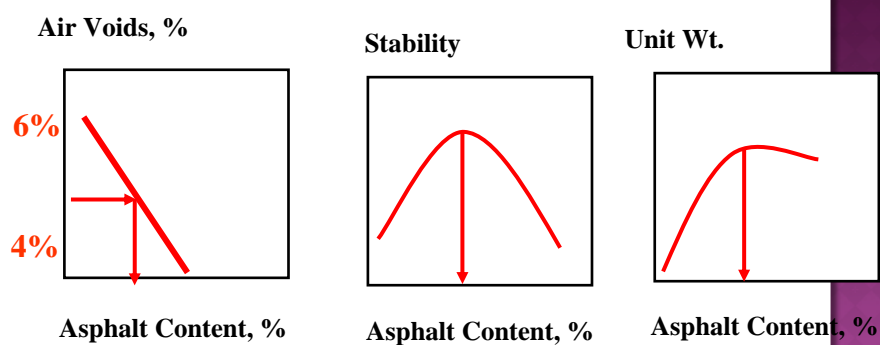




# Asphalt Content Criteria

- Highest Stability
- Highest Unit Weight
- Median Void Content

## MARSHALL DESIGN USE OF DATA ASPHALT INSTITUTE PROCEDURE

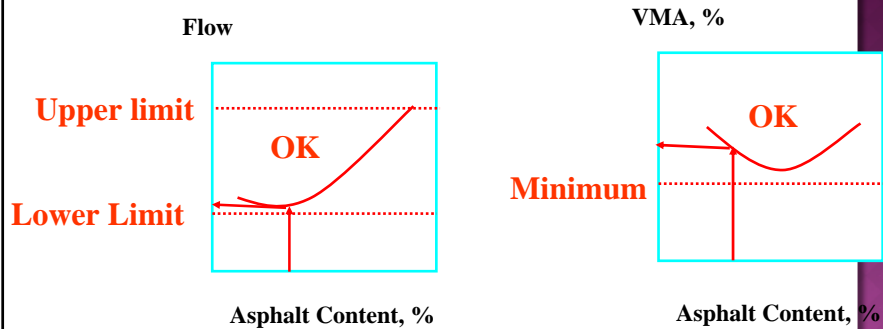


*Target optimum asphalt content = average*

# Summary

Characteristic	% Asphalt
Highest Stability	5.8
Highest Unit Weight	4.7
Optimum Voids Content	5.3

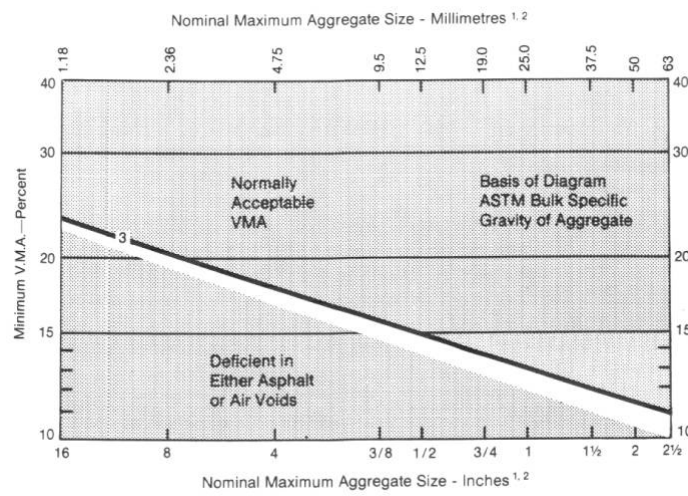
## MARSHALL DESIGN USE OF DATA ASPHALT INSTITUTE PROCEDURE



*Use target optimum asphalt content to  
check if these criteria are met*

Characteristic	% Asphalt
Highest Stability	5.8
Highest Unit Weight	4.7
Optimum Voids Content	5.3
<b>Total</b>	<b>15.8</b>
	<b>3</b>
<b>Average:</b>	<b>5.3%</b>

## MINIMUM VMA REQUIREMENTS





## MARSHALL DESIGN METHOD

### Advantages

- Attention on voids, strength, durability
- Inexpensive equipment
- Easy to use in process control/acceptance

### Disadvantages

- Impact method of compaction
- Does not consider shear strength
- Load perpendicular to compaction axis
- Does not predict performance

## MARSHALL DESIGN CRITERIA

	Light Traffic ESAL < 10 <sup>4</sup>	Medium Traffic 10 <sup>4</sup> < ESAL < 10 <sup>5</sup>	Heavy Traffic ESAL > 10 <sup>5</sup>
<b>Compaction</b>	35	50	75
<b>Stability N (lb.)</b>	3336 (750)	5338 (1200)	8006 (1800)
<b>Flow, 0.25 mm (0.1 in)</b>	8 to 18	8 to 16	8 to 14
<b>Air Voids, %</b>	3 to 5	3 to 5	3 to 5
<b>Voids in Mineral Agg. (VMA)</b>	Varies with aggregate size		

	Design Criteria (Heavy Traffic)		Values at 5.3% Asphalt Content
	Min.	Max.	
Stability (lbs)	1500	—	2080
Flow (0.01")	8	16	13
% Air Voids	3	5	4

