

Homework # 6

ASSHO Method

Assume SN = 4.0

given $P_t = 2.5$

Axle Load (Kips)	Single Axle			Tandem Axle		
	Repletion (N)	EWLF (F)	F*N	Repletion (N)	EWLF (F)	F*N
2	249	0.0003	0.07			
6	525	0.02	10.50			
10	1051	0.12	126.12			
14	249	0.40	99.60			
18	182	1.00	182.00			
20	140	1.49	208.60			
22	90	2.17	195.30			
26	10	4.31	43.10			
10				156	0.01	1.56
16				168	0.07	11.76
22				177	0.23	40.71
28				292	0.55	160.60
32				103	0.89	91.67
34				111	1.11	123.21
38				21	1.69	35.49
42				4	2.49	9.96
Total			865.29			474.96

$$EAL_o = 865.29 + 474.96 = 1340.25 \text{ Axle/day}$$

$$\sum_0^n EAL = \frac{EAL_o (365)}{\log_e (1+i)} [(1+i)^n - 1]$$

$$\sum EAL = \frac{1340.25 * 365}{0.04879} [(1.05)^{10} - 1] = 6,305,569.04$$

$$\text{Total } EAL_{10} \text{ for Pavement design lane} = 0.6 * 0.6 * 6.31 * 10^6 = 2.3 * 10^6$$

Assume: R = 95%, $S_o = 4.5$, $P_o = 5.0$, a and no loss due to forest

$$\text{Then: } \Delta PSI = 5.0 - 2.5 = 2.5$$

Assume that bituminous treated layers will be used, therefore, for the three layers we can found MR from Marshall Stability from Fig. 2.9. as follow:

Layer	Marshall Stability (lbs)	MR (psi)
Wearing Course	1900	$4.4 * 10^5$
Base	1400	$3.0 * 10^5$
Subbase	1000	$1.9 * 10^5$

From figure 2.5 $\Rightarrow a_1 = 0.43$

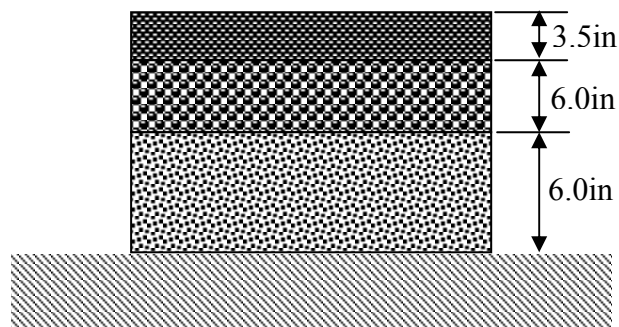
From figure 2.9 $\Rightarrow a_2 = 0.26$

From figure 2.5 $\Rightarrow a_3 = 0.19$

Subgrade $M_R = 15000$ psi $\Rightarrow SN = 3.0$ (fig. 3.1)

Assume $D_{WC} = 3.5$ in (The Minimum), $D_{base} = 6.0$ in (The Minimum), $D_{subbase} = 5.0$ in

Therefore $\Rightarrow SN = 3.5 * 0.43 + 6.0 * 0.26 + 6.0 * 0.19 = 4.21$



MOC Method

By calculating EWLF from fig. 1.09.2, the following table was obtained:

Axle Load (Ton)	Single Axle			Tandem Axle		
	Repletion (N)	EWLF (F)	F*N	Repletion (N)	EWLF (F)	F*N
0.9	249	0.0000	0.00			
2.7	525	0.03	15.75			
4.5	1051	0.09	94.59			
6.4	249	0.52	129.48			
8.2	182	1.10	200.20			
9.1	140	1.40	196.00			
10.0	90	2.24	201.60			
11.8	10	4.32	43.20			
4.5				156	0.01	1.56
7.3				168	0.04	5.88
10.0				177	0.18	31.86
12.7				292	0.43	125.56
14.5				103	0.81	83.43
15.4				111	1.10	122.10
17.2				21	1.50	31.50
19.1				4	2.20	8.80
Total			880.82			410.69

$$EAL_0 = 880.82 + 410.69 = 1291.51$$

$$G.F. = \left(1 + \frac{AG}{100}\right)^{10} = (1.05)^{10} = 1.63$$

$$EAL_{10} = EAL_0 * G.F = 1291.51 * 1.63 = 2103.73$$

$$\text{Total } EAL_{10} = \left(\frac{EAL_0 + EAL_{10}}{2}\right) * n * 365 = \frac{1291.51 + 2103.73}{2} * 10 * 365 = 6,196,319.75$$

$$\text{Total } EAL_{10} \text{ for Pavement design lane} = 0.6 * 0.6 * 6.2 * 10^6 = 2.2 * 10^6$$

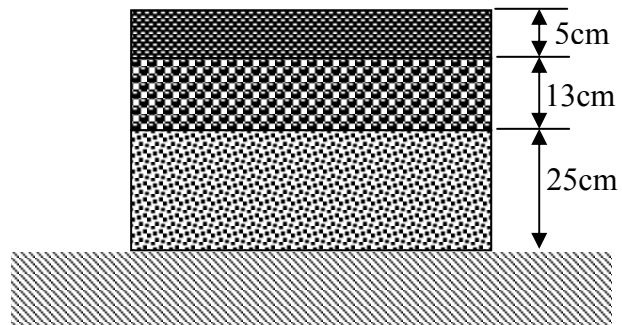
$$MR = 1500 * CBR \Rightarrow CBR \text{ for the Subgrade} = 15000 / 1500 = 10$$

Minimum cover depth require = 43 cm (from fig. 1.09.8)

Form Figure 1.09.5

AC base course thickness = 12.5 cm

Therefore, we can use 5cm AC surface + 13 cm AC base + 25 cm crushed aggregate subbase. The total cover is 43 cm. OK.



Asphalt Institute method

From Table IV - 4

Axle Load (lb)	Single Axle			Tandem Axle		
	Repletion (N)	EWLF (F)	F*N	Repletion (N)	EWLF (F)	F*N
2000	249	0.0002	0.04			
6000	525	0.01	5.48			
10000	1051	0.09	92.17			
14000	249	0.36	89.64			
18000	182	1.00	182.00			
20000	140	1.51	211.40			
22000	90	2.18	196.20			
26000	10	4.09	40.90			
10000				156	0.00688	1.07
16000				168	0.05	7.93
22000				177	0.18	31.86
28000				292	0.50	144.54
32000				103	0.86	88.27
34000				111	1.10	121.55
38000				21	1.70	35.70
42000				4	2.51	10.04
Total			817.83			440.96

$$EAL_o = 817.83 + 440.96 = 1258.78$$

$$\sum EAL = \frac{1258.78 * 365}{0.04879} [(1.05)^{10} - 1] = 5,922,291.33$$

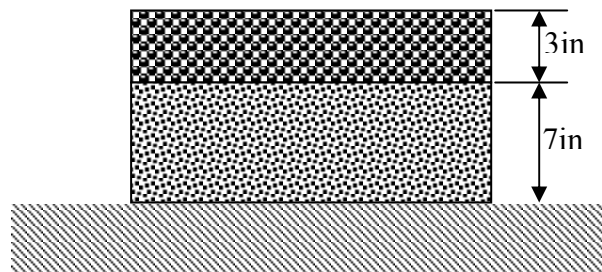
$$\text{Total } EAL_{10} \text{ for Pavement design lane} = 0.6 * 0.6 * 5.92 * 10^6 = 2.1 * 10^6$$

From Chart VI-11:

Full depth asphalt concrete = 9in

Assume AC surface = 3in (Minimum, Table VI-2)

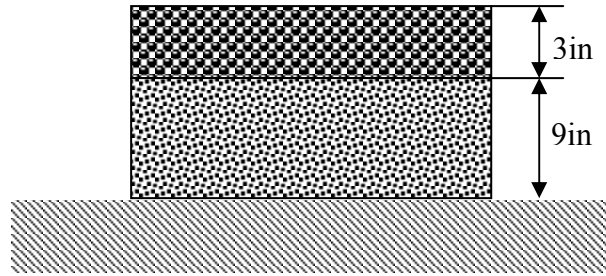
Therefore, AC base = 6 in



From Chart VI-13:

Thickness of the asphalt surface and type II base = 11in

Assume AC surface = 3 in
Therefore, Emulsified asphalt base = 9in



From design chart VI-16
Thickness of asphalt surface and base = 9in
Plus 6.0in Granular Base

