

Chapter 2.

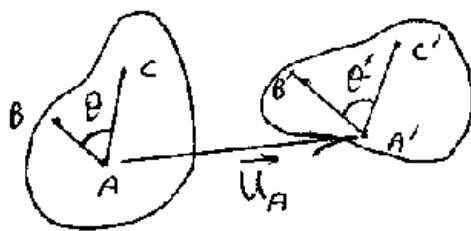
Deformation

2.1 Deformation

Deformation is the change in volume and shape of bodies when subjected to forces.

Deformation is noticeable for materials like rubber whereas it is hard to be seen for structural members like reinforced concrete --- why?

Displacement : is a vector quantity which measures the movement of a particle or a point in a body from one position to another.



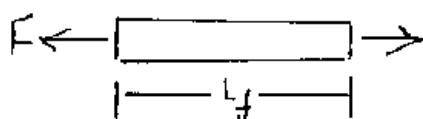
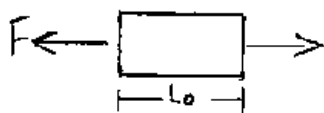
$AB' \text{ may } \neq AB$
 $AC' \text{ may } \neq AC$
 $B'C' \text{ may } \neq BC$
 $\theta' \text{ may } \neq \theta$

\vec{u}_A is the displacement vector of point A.

The best way to quantify deformation is to measure strains

2.2 Strains

As there are two kinds of stresses, there are two kinds of strain. Normal strains caused by normal stresses and shear strains caused by shear stresses.



$L_0 =$ original length

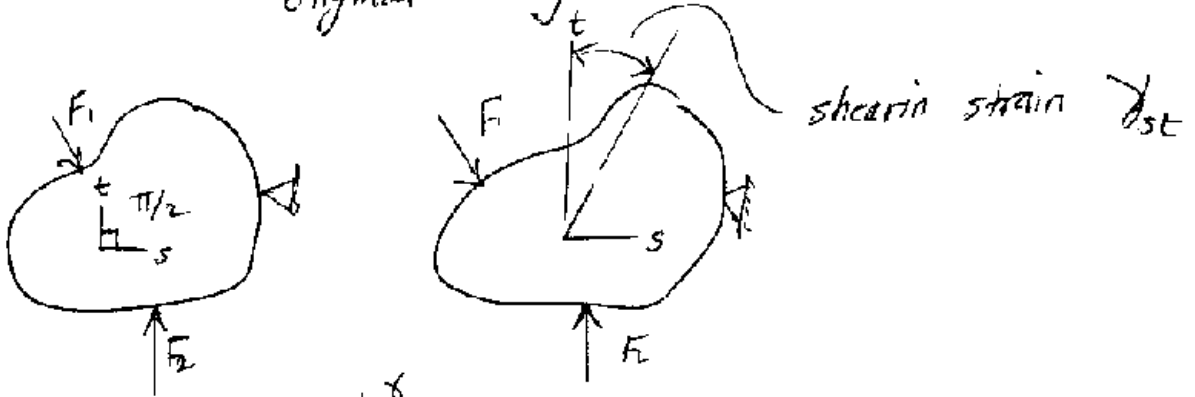
$L_f =$ final length

$\Delta L = \delta =$ change in length

$$\text{Normal strain} = \frac{\text{change in length}}{\text{original length}} \quad \left(\epsilon = \frac{\Delta L}{L_0} = \frac{\delta}{L_0} \right)$$

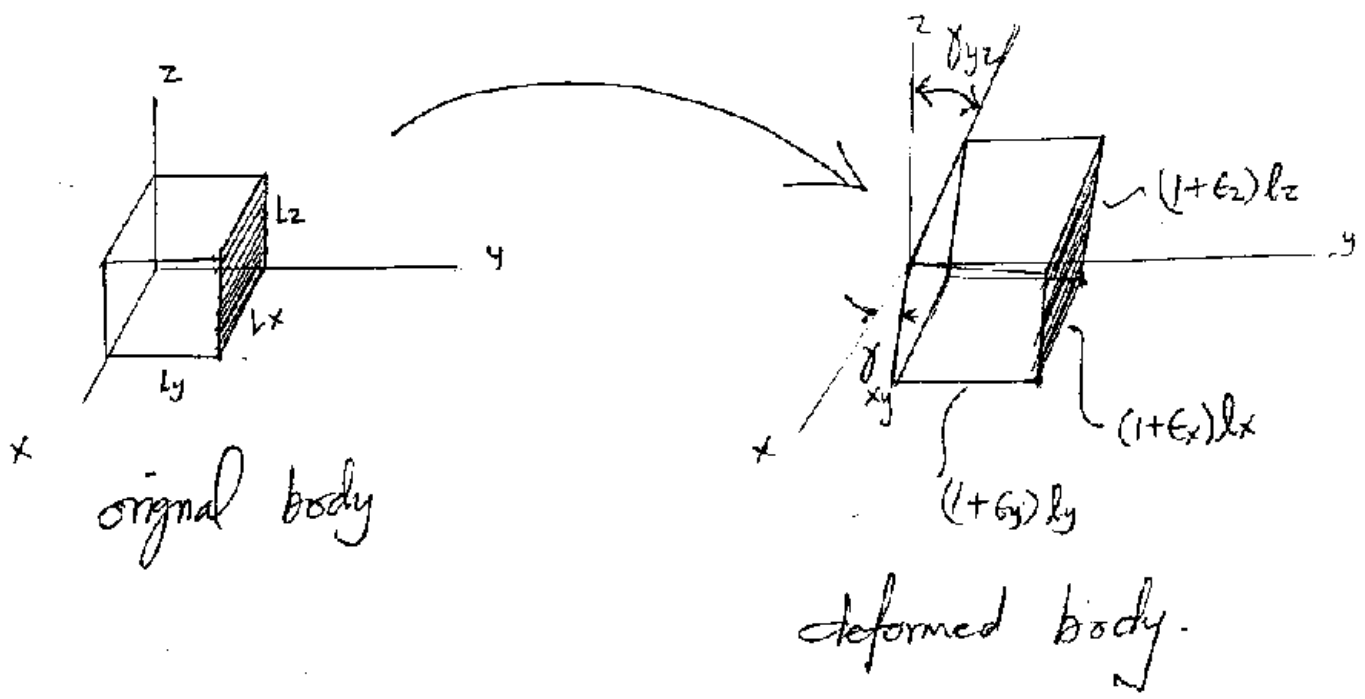
$$L_f = (1 + \epsilon) L_0$$

Shear strain: the change in the 90° angle in the original body.



as $\gamma \ll 1$
 $\therefore \sin \gamma = \gamma$
 $\cos \gamma = 1$
 $\& \tan \gamma = \frac{\sin \gamma}{\cos \gamma} = \gamma$

Change of shape and volume



3.1 The Tension and Compression Test

One of the most important test to perform is the tension or compression test which is meant to establish the relationship between normal stress σ and normal strain ϵ using a standard specimen.

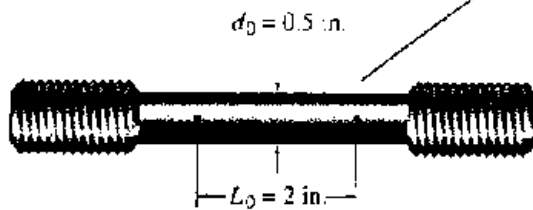


Fig. 3-1

① Gauge length L_0 is the length along which strain is monitored.

② Extensometer is a device to measure change in length

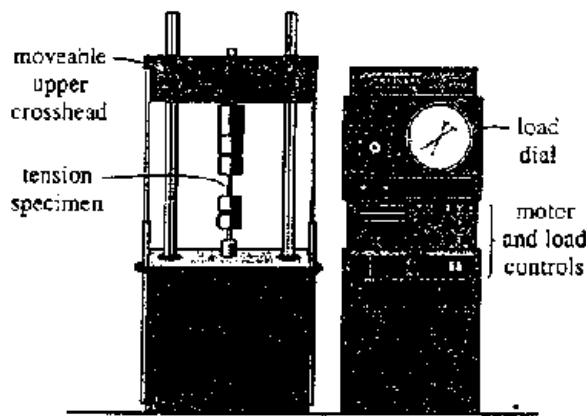


Fig. 3-2



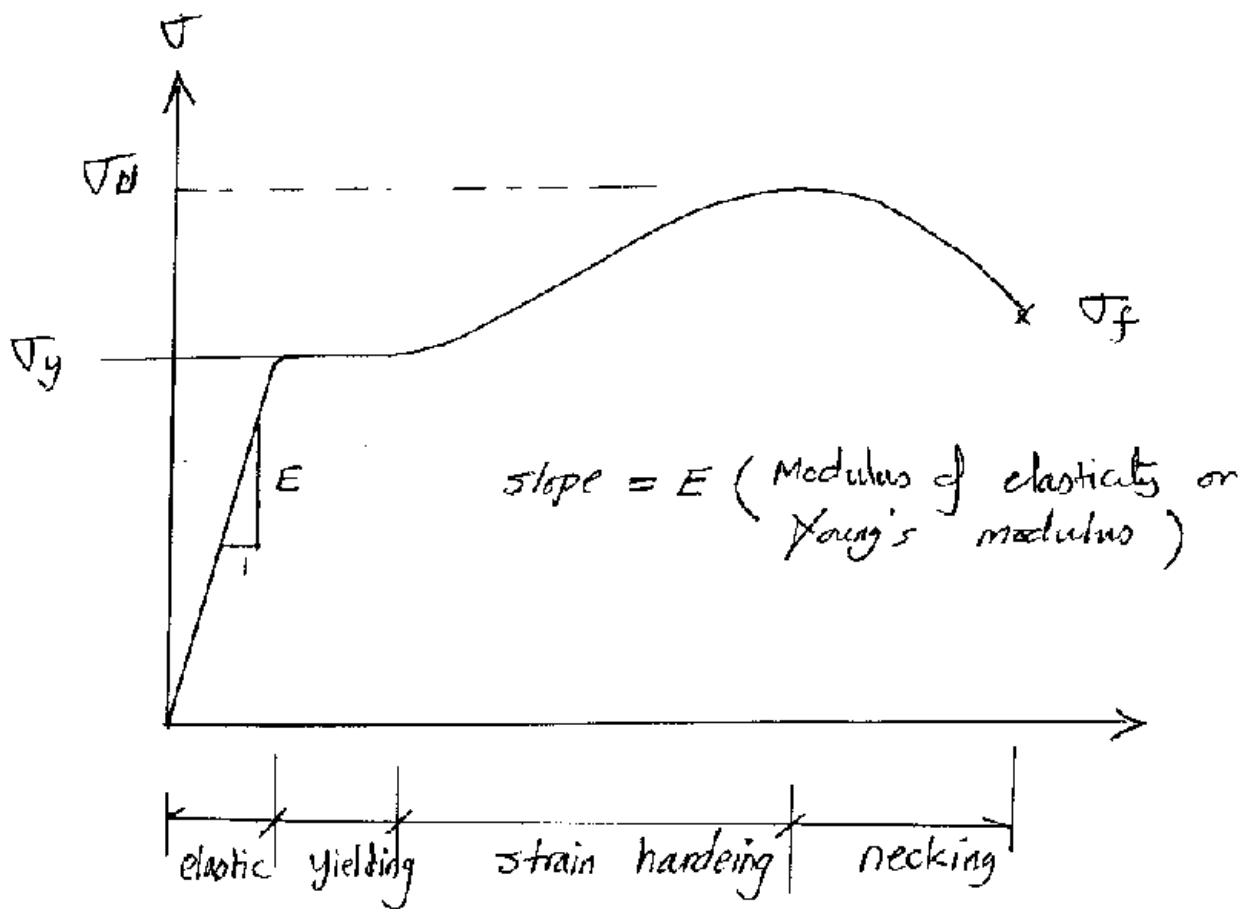
strain gage: an electrical device to measure strain (normal strain) directly

The end product of the test is a stress-strain curve from which material properties can be calculated.

3-2 The Stress - Strain Diagram.

Using a standard specimen of known gage length L_0 and of original cross section area A_0 and run a tension test according to ASTM (American Society for Testing and Materials) specifications. This required applying a force P with a specified rate then.

- Calculate normal stress $\sigma = \frac{P}{A_0}$
 - Calculate normal strain $\epsilon = \Delta L / L_0$
 - plot stress versus strain,
- one may have a curve like this



stress-strain curve for ductile steel.

3.3 Stress - Strain Behavior of Ductile and Brittle materials

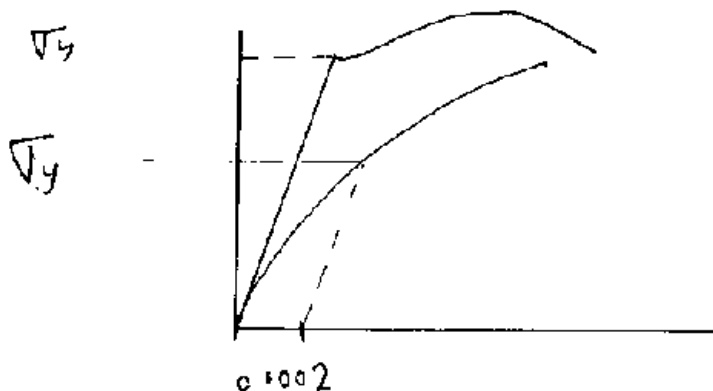
Ductile material : are those materials which exhibit large deformation before failing.

Ductility is measured by one of two :

$$\% \text{ elongation} = \frac{L_f - L_0}{L_0} \times 100$$

$$\% \text{ reduction in area} = \frac{A_0 - A_f}{A_0} \times 100$$

For material which does not possess definite yield point, the yield stress (strength) is obtained by the use of offset method which is normally at 0.002 strain.



Brittle materials : are those which exhibit little or no yielding before failing

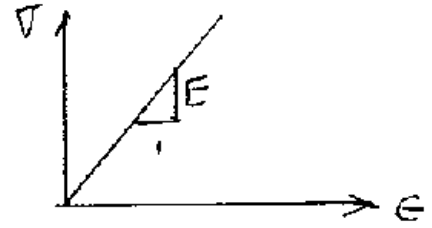
examples { Concrete and cast iron }

See Fig. 3.9 and Fig 3.11 in your text book pages (92 & 93)

3.4 Hooke's Law.

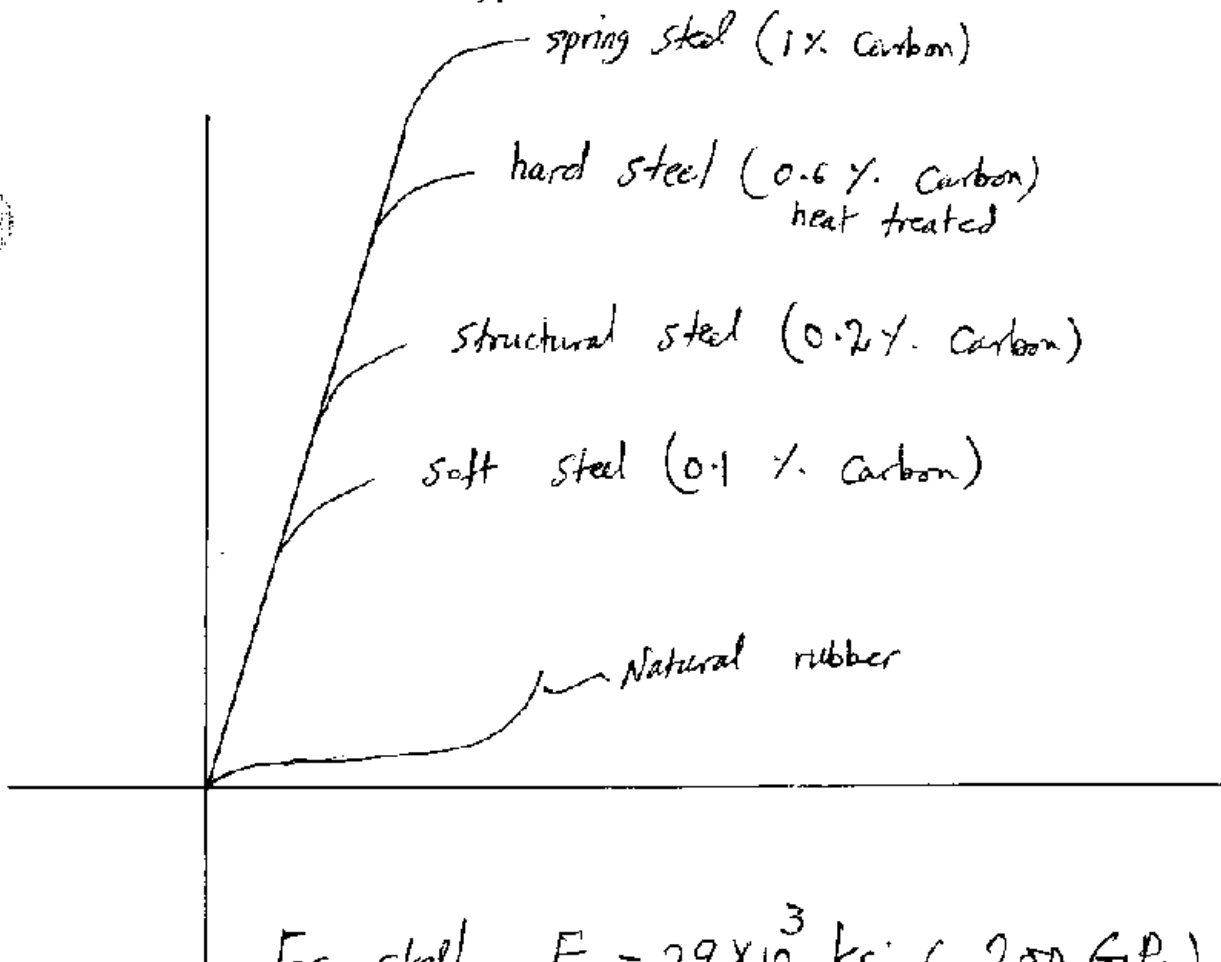
The linear stress-strain relationship is called Hooke's law.

$$\sigma = E \epsilon$$



Remember E is called modulus of elasticity.

Remark: Although one may vary the yield strength of steel by varying the % of carbon, the modulus of elasticity remain the same as in steel.

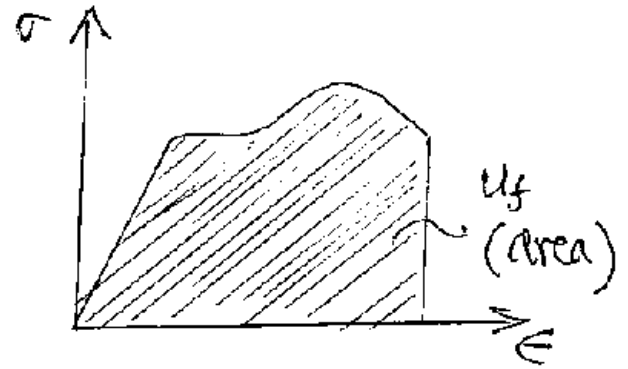
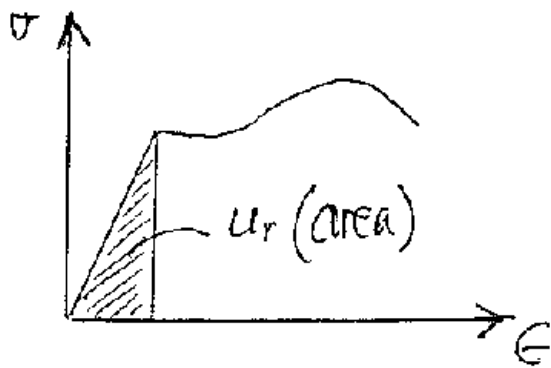


For steel $E = 29 \times 10^3 \text{ ksi}$ (200 GPa)

For rubber $E = 0.1 \times 10^3 \text{ ksi}$ (0.7 MPa)

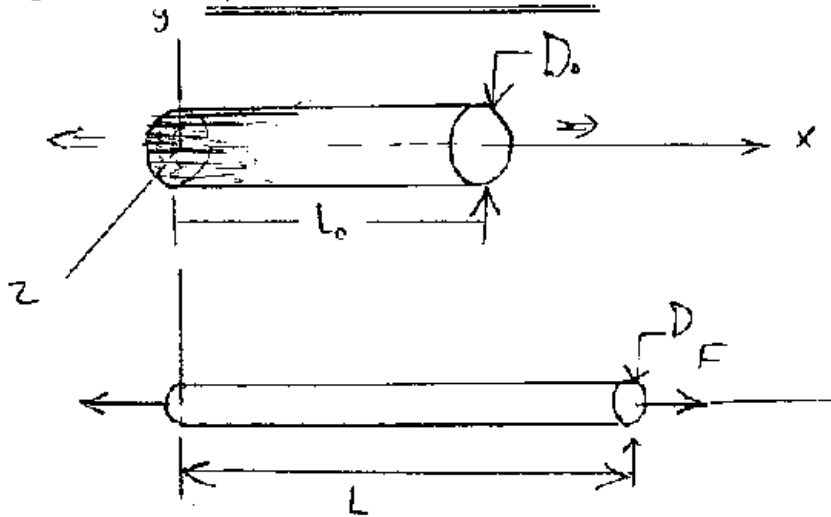
That is why deformation in rubber is noticeable.

There are two more important properties for materials one is called Modulus of Resilience U_r and the other is modulus of toughness U_f



3.6 Poisson's Ratio ν

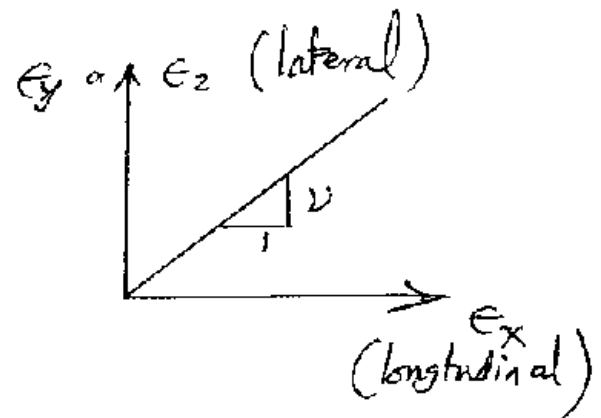
- L_0 = original length
- D_0 = original diameter
- D = current diameter
- L = current length



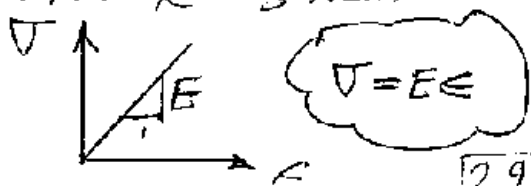
$$\epsilon_x = \frac{L - L_0}{L} > 0$$

$$\epsilon_y = \epsilon_z = \frac{D - D_0}{D_0} < 0$$

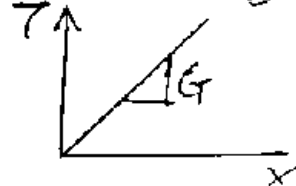
$$\epsilon_y \text{ or } \epsilon_z = -\nu \epsilon_x$$



Poisson's ratio ν is nothing but the relation between lateral strain to the longitudinal strain



$$\nu = E \epsilon$$



$$\tau = G \gamma$$

Modulus of rigidity

Average Mechanical Properties of Typical Engineering Materials^a
(U.S. Customary Units)

Materials	Specific Weight γ (lb/in. ³)	Modulus of Elasticity E (10 ³) ksi		Modulus of Rigidity G (10 ³) ksi		Yield Strength (ksi)		Ultimate Strength (ksi)		% Elongation in 2 In. Specimen	Poisson's Ratio ν	Coef. of Therm. Expansion α (10 ⁻⁶)/°F		
		Resiliently	Plastically	Tens.	Comp. ^b	Tens.	Comp. ^b	Tens.	Comp. ^b				Tens.	Comp. ^b
Metallic	0.101	Aluminum [2014-T6 Wrought Alloys]-6061-T6		10.6	7.5	60	90	25	68	68	42	0.35	12.8	
		Cast Iron [Gray ASTM 20 Alloys]-Malleable ASTM A-197		10.0	3.3	37	37	19	42	42	27	12	0.35	13.1
		Copper [Red Brass C83400 Alloys]-Bronze C86100		55.0	9.8	—	—	—	—	—	—	0.6	0.28	6.70
		Magnesium [Am 1004-T651] Alloy		14.6	5.4	11.4	11.4	—	35	35	—	35	0.35	6.60
		Steel [Structural A36 Alloys]-Stainless 304		15.0	5.6	30	50	—	95	95	—	28	0.34	9.80
		Titanium [Ti 6Al 4V] Alloy		6.48	2.5	22	32	—	43	40	32	1	0.39	14.3
		Concrete [Low Strength High Strength]		23.0	1.0	36	36	—	58	58	—	30	0.32	6.00
		Plastic [Kevlar 49 Reinforced]-30% Glass		28.0	1.0	30	30	—	75	75	—	40	0.27	9.60
		Wood [Select Structural Grade] [Douglas Fir White Spruce]		39.0	—	102	102	—	116	116	—	37	0.33	6.30
		Titanium [Ti 6Al 4V] Alloy		12.4	6.4	174	134	—	145	145	—	16	0.36	5.20
Nonmetallic	0.086	Concrete [Low Strength High Strength]		3.20	—	—	—	1.8	—	—	—	0.15	6.0	
		Plastic [Kevlar 49 Reinforced]-30% Glass		4.20	—	—	—	5.5	—	—	—	—	0.15	6.0
		Wood [Select Structural Grade] [Douglas Fir White Spruce]		9.0	—	—	—	—	104	70	10.2	2.8	0.34	—
		Wood [Select Structural Grade] [Douglas Fir White Spruce]		9.3	—	—	—	—	13	19	—	—	0.34	—
Wood [Select Structural Grade] [Douglas Fir White Spruce]		0.017	1.80	—	—	—	0.40 ^c	3.70 ^d	6.90 ^d	—	0.29 ^e	—		
Wood [Select Structural Grade] [Douglas Fir White Spruce]		0.130	1.40	—	—	—	0.36 ^e	2.18 ^e	0.91 ^e	—	0.31 ^e	—		

^a Specific values may vary for a particular material due to alloy or mineral composition, mechanical working of the specimen, or heat treatment. For a more exact value reference books for the material should be consulted.

^b The yield and ultimate strengths for ductile materials can be assumed equal for both tension and compression.

^c Measured perpendicular to the grain.

^d Measured parallel to the grain.

^e Deformation measured perpendicular to the grain when the load is applied along the grain.

Average Mechanical Properties of Typical Engineering Materials*
(U.S. Customary Units)

Material	Specific Weight γ (lb/in. ³)	Modulus of Elasticity E (10 ⁶ psi)		Tens. Comp. ^b Shear		Ultimate Strength (ksi)		# Specimens in Tension	Poisson's Ratio ^c	Coeff. of Thermal Expansion α (in./in./°F)
		(10 ⁶ psi)	(10 ⁶ ksi)	Tens. Comp. ^b	Shear	Tens. Comp. ^b	Shear			
Metals	0.101	10.5	3.0	40	40	41	10	0.35	17.4	
				37	37	37	12	0.35	13.1	
				—	—	—	0.5	0.26	0.70	
				—	—	—	1	0.28	0.60	
				11.4	11.4	—	0.5	0.35	0.50	
				30	30	—	20	0.34	0.60	
				32	32	—	1	0.30	14.7	
				36	36	—	30	0.32	0.60	
				30	30	—	40	0.27	0.60	
				102	102	—	31	0.32	0.50	
Metals	0.168	17.4	6.6	138	134	—	10	0.36	1.70	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
Concrete	0.150	4.20	—	—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
				—	—	—	—	—	—	
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				—	—	—	—	—	—	
				—	—	—	—	—	—	
Plastics	0.017	0.50	—	—	—	—	—	—	—	
				—	—	—	—	—	—	
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				—	—	—	—	—	—	

* Specific values may vary for a particular material due to alloy or mineral composition, mechanical working of the specimen, or heat treatment. For a more exact value reference books for the material should be consulted.

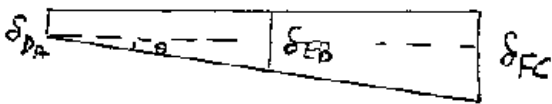
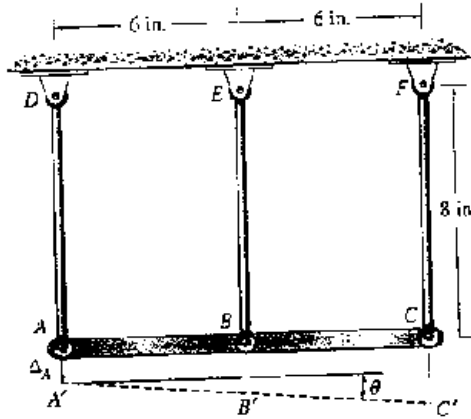
^b The yield and ultimate strengths for ductile materials can be assumed equal for both tension and compression.

^c Measured perpendicular to the grain.

^d Measured parallel to the grain.

^e Deformation measured perpendicular to the grain when the load is applied along the grain.

2-3. Bar ABC is originally in a horizontal position. If loads cause the end A to be displaced downwards $\Delta_A = 0.002$ in. and the bar rotates $\theta = 0.2^\circ$, determine the average normal strain in the rods AD, BE, and CF.



$$\delta_{DA} = 0.002 \text{ in}$$

$$\tan \theta = \frac{\delta_{EB} - 0.002}{6} = \tan 0.2^\circ \Rightarrow \delta_{EB} = 0.02294 \text{ in}$$

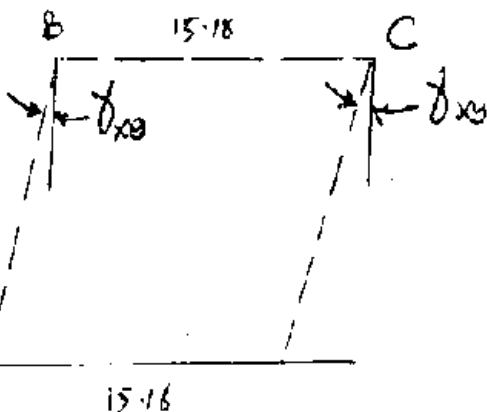
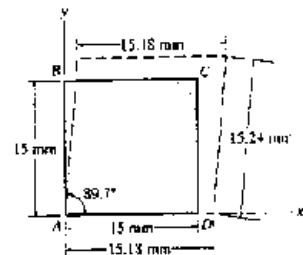
$$\tan \theta = \frac{\delta_{FC} - 0.002}{12} = \tan 0.2^\circ \Rightarrow \delta_{FC} = 0.04388 \text{ in}$$

$$\epsilon_{AD} = \frac{\delta_{AD}}{l_{AD}} = \frac{0.002}{8} = 2.5 \times 10^{-4} \text{ in/in}$$

$$\epsilon_{BE} = \frac{\delta_{BE}}{l_{BE}} = \frac{0.02294}{8} = 2.8675 \times 10^{-3} \text{ in/in}$$

$$\epsilon_{FC} = \frac{\delta_{FC}}{l_{FC}} = \frac{0.04388}{8} = 5.485 \times 10^{-3} \text{ in/in}$$

2-22. A square piece of material is deformed into the dashed position. Determine the shear strain γ_B at corners B and C.



$$\gamma_{xy} = -\frac{0.3^\circ \pi}{180} \text{ at B}$$

$$\gamma_{xy} = +\frac{0.3^\circ \pi}{180} \text{ at C}$$

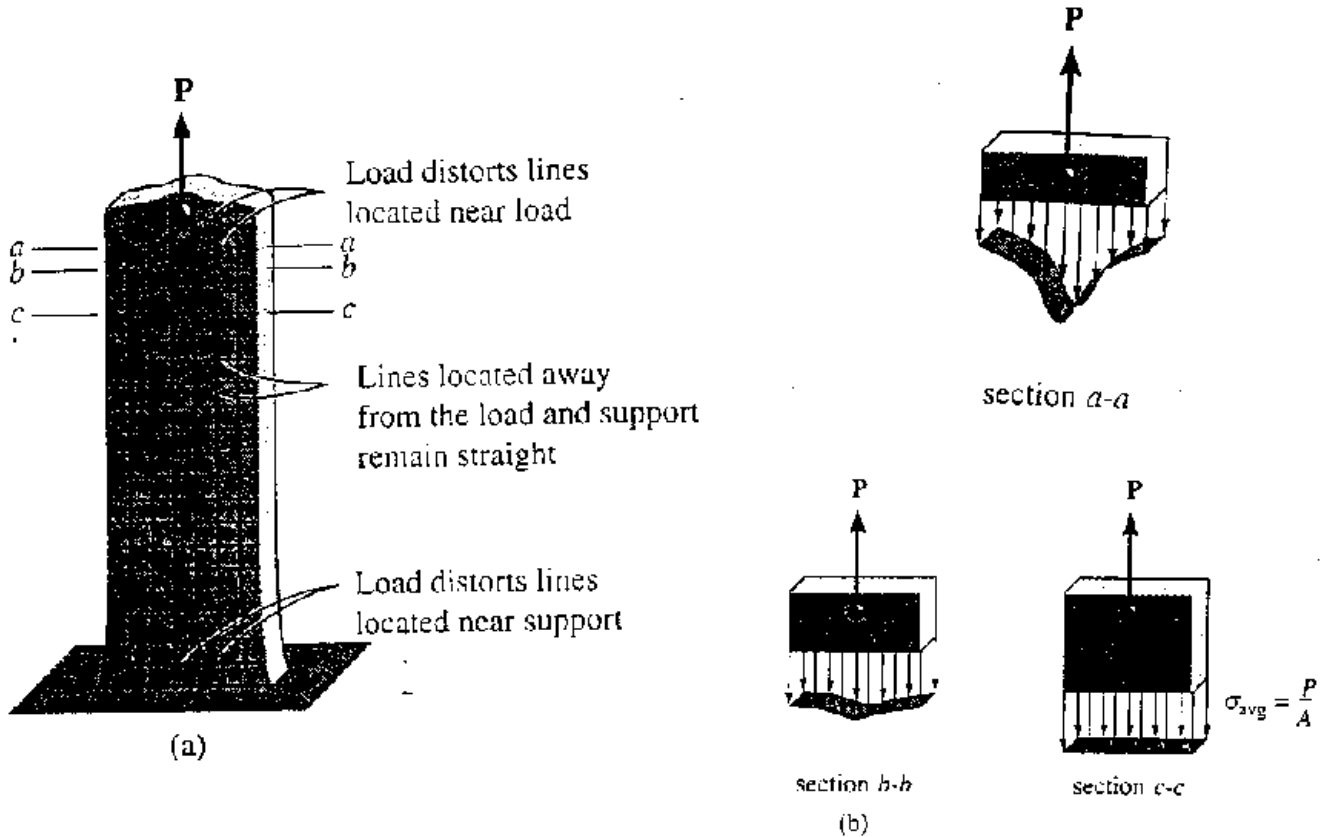
both of which = 5.24×10^{-3} rad
 $\gamma = \tan 0.3^\circ = 5.24 \times 10^{-3}$ rad

2-22
79

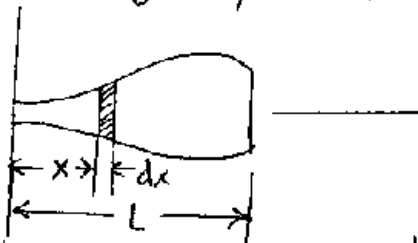
Ch. 4 Axial Load

4.1

Saint-Venant Principal: The stress tends to be large and non-uniform near or at support and where the load is applied and the stress tends to flatten and becomes uniform at a distance equal to the largest dimension away from support and load application.



Just like stress, strain is define at a point which may varies and change from point to point.



strain of element dx $\epsilon(x) = \frac{d\delta}{dx}$ and

$d\delta$ = elongation of element dx
 δ = elongation of the whole member

$$\delta = \int_0^L \epsilon(x) dx$$

for constant stress and strain $\epsilon(x) = \epsilon = \text{constant}$ and the above becomes

$$\underline{\underline{\delta = \epsilon L}}$$

For a member made of material whose modulus of elasticity E :

$$\sigma(x) = E \epsilon(x) \quad \text{--- Hooke's law}$$

$$\frac{N(x)}{A(x)} = E \epsilon(x) \quad \text{--- for variable load and area}$$

$\therefore \epsilon(x) = \frac{N(x)}{E A(x)}$, which upon substitution,

we have:

$$\delta = \int_0^L \frac{N(x)}{E A(x)} dx$$

For constant $N(x)$ and $A(x)$: $N(x) = N$, $A(x) = A \Rightarrow$

$$\delta = \frac{NL}{EA}$$

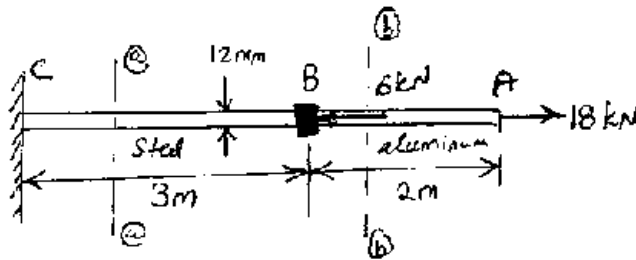
, for member composed of different

materials or different segments of different areas, then

$$\delta = \sum_{i=1}^n \frac{N_i L_i}{E_i A_i}$$

where n is number of segments.

Prob. 4-1
30



$$E_{st} = 200 \text{ GPa}$$

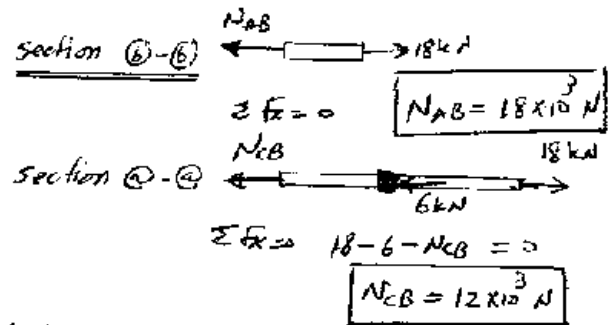
$$E_{al} = 70 \text{ GPa}$$

Determine displacement of B and the end A.

$$\delta_{Bc} = \delta_{Bc} = \frac{N_{CB} l_{CB}}{E_{st} A_{CB}} = \frac{(12 \times 10^3)(3)}{(200 \times 10^9) \left(\frac{\pi (0.012)^2}{4} \right)}$$

$$\delta_{Bc} = 0.00159 \text{ m}$$

$$= 1.59 \text{ mm}$$



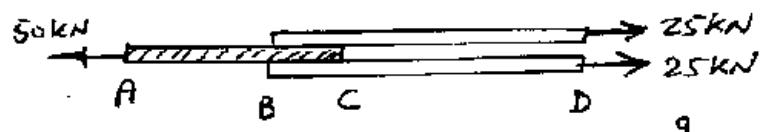
$$\delta_A = \delta_{CB} + \delta_{BA}$$

$$= 0.00159 + \frac{N_{AB} l_{AB}}{E_{al} A_{AB}} = 0.00159 + \frac{(18 \times 10^3)(2)}{(70 \times 10^9) \left(\frac{\pi (0.012)^2}{4} \right)}$$

$$= 0.00614 \text{ m}$$

$$= 6.14 \text{ mm}$$

4-8
128



A-36 steel $\Rightarrow E = 200 \times 10^9$
 $L_{AB} = 600 \text{ mm}$
 $L_{BC} = 200 \text{ mm}$
 $L_{CD} = 800 \text{ mm}$
 $\delta_{A/D} = ?$

$\delta = \frac{NL}{EA}$

$$\delta_{A/D} = \delta_{A/B} + \delta_{B/C} + \delta_{C/D}$$

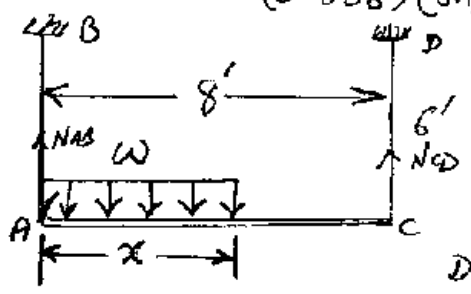
$$= \frac{N_{AB} L_{AB}}{E A_{AB}} + \frac{N_{BC} L_{BC}}{E A_{BC}} + \frac{N_{CD} L_{CD}}{E A_{CD}}$$

$$= \left\{ \frac{(50 \times 10^3)(0.6)}{(0.006)(0.1)} + \frac{(50 \times 10^3)(0.2)}{3(0.006)(0.1)} + \frac{(50 \times 10^3)(0.8)}{2(0.006)(0.1)} \right\} \frac{1}{29 \times 10^6}$$

$$= \frac{(50 \times 10^3) \{0.6 + 0.2/3 + 0.8/2\}}{(0.006)(0.1) 200 \times 10^9} = 4.44 \times 10^{-4} \text{ m}$$

$$= \boxed{0.444 \text{ mm}}$$

4-21
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AB & CD are steel wires
 $d_{AB} = 0.5 \text{ in}$, $d_{CD} = 0.3 \text{ in}$
 $\sigma_{all} = 16.2 \text{ ksi}$

Determine w & x such that AC remains horizontal.

$$\sum F_x = 0: N_{AB} + N_{CD} = wx \quad \text{--- (1)}$$

$$\sum M_{@A} = 0: 8N_{CD} = \frac{wx^2}{2} \quad \text{--- (2)}$$

AC remains horizontal: $\delta_{AB} = \delta_{CD}$

$$\frac{N_{AB}(8)}{\pi(0.5)^2} = \frac{N_{CD}(6)}{\pi(0.3)^2} \quad \text{--- (3)}$$

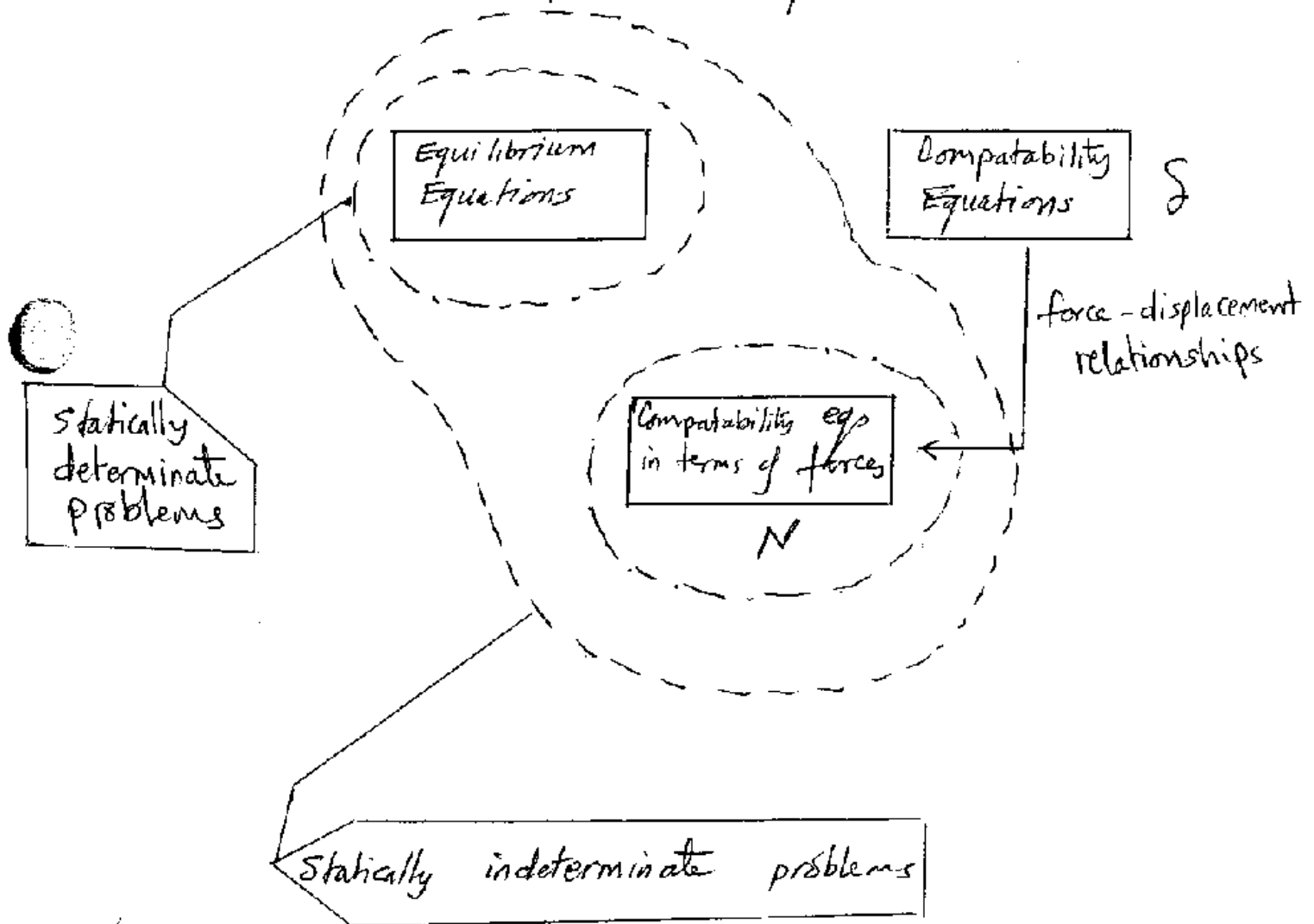
$$\sigma_{CD} = \sigma_{all}: \frac{N_{CD}}{\pi(0.3)^2} = 16.2 \times 10^3 \quad \text{--- (4)}$$

From (4): $N_{CD} = 1145 \text{ lb}$
 From (3): $N_{AB} = 3172 \text{ lb}$
 From (1) & (2): $16N_{CD} = (N_{AB} + N_{CD})x \Rightarrow \underline{x = 4.24 \text{ ft}}$
 from (1): $\underline{w = 1.02 \text{ kip/ft}}$

Check that $\sigma_{AB} \leq \sigma_{all}$: $\frac{N_{AB}}{\pi(0.5)^2} \leq 16.2 \times 10^3 \quad \underline{\text{OK}}$

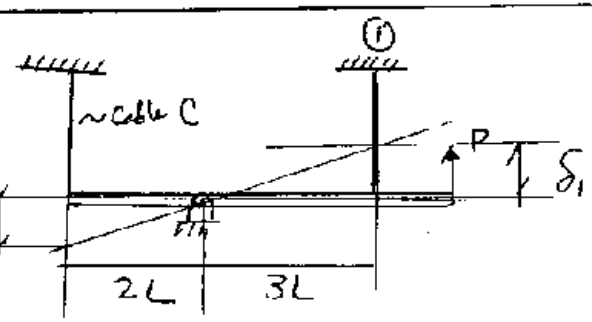
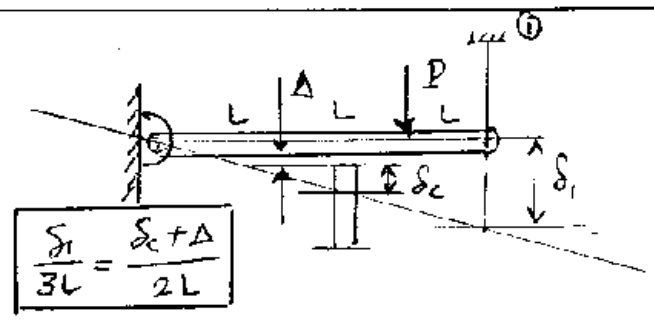
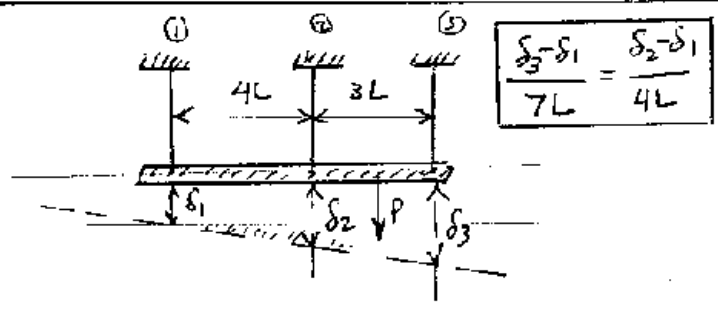
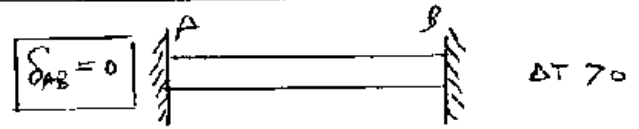
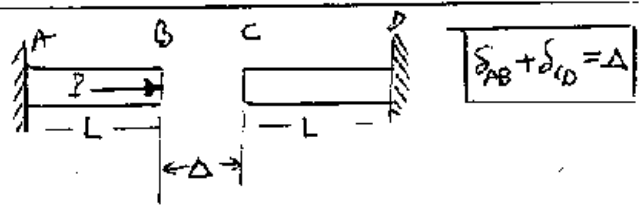
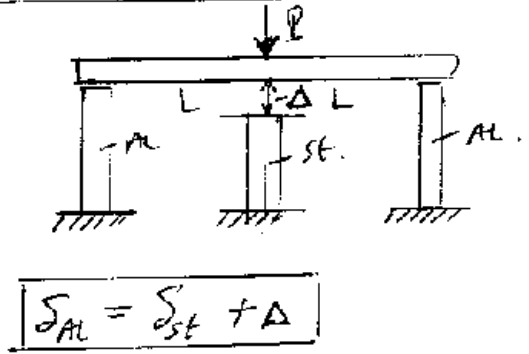
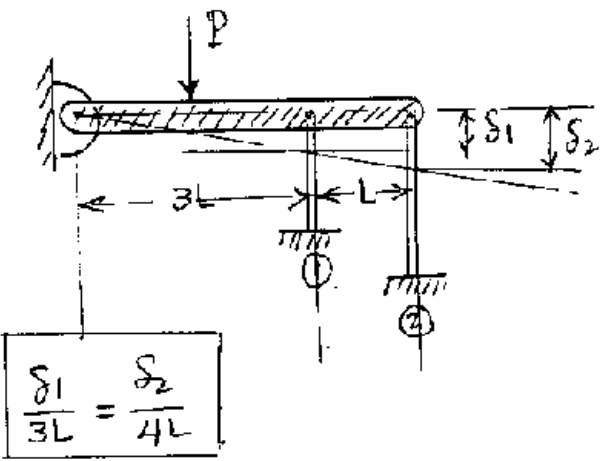
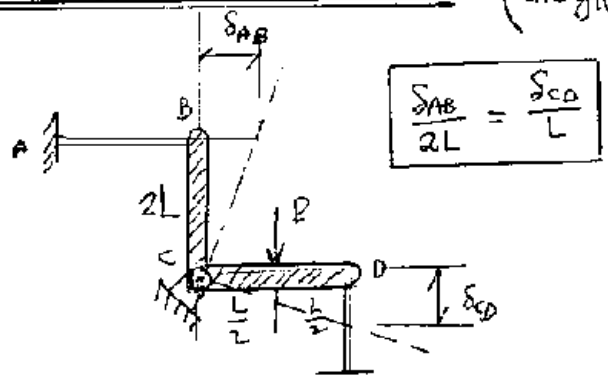
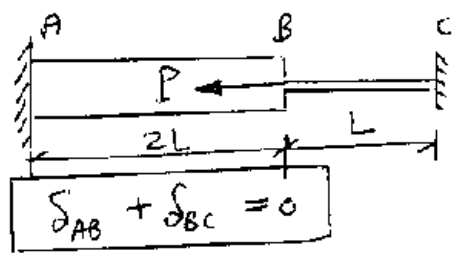
Statically Indeterminate of Axially loaded Members

When number of unknowns exceeded number of equations of equilibrium, we say that the structure is statically indeterminate and thus the unknowns (reactions or internal normal forces) can not be obtained from equations of equilibrium alone and the one needs additional equations. These equations can be obtained from the deformation of the structure. These extra equations are known as compatibility equations.

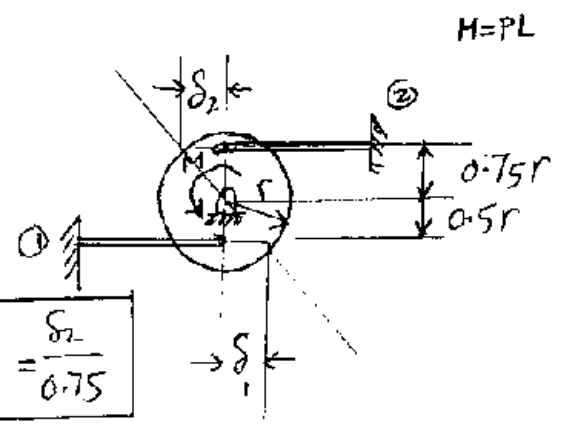


Compatibility equations are nothing but relationships between displacement of members which will be more useful if converted in terms of internal unknown forces which are used with eq. eqs to solve for unknowns.

Examples of Statically Indeterminate Problems (Part L are given)

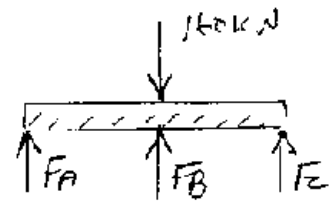
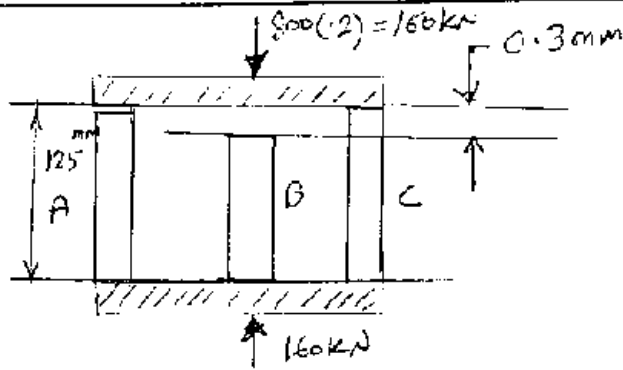


$$\frac{\delta_1}{3L} = \frac{\delta_c}{2L}$$



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$E = 70 \times 10^9 \text{ Pa}$
 $A = 400 \text{ mm}^2$



unknowns $\{F_A, F_B, F_C\}$

Equilibrium Equations:

① $\sum M_{@B} = 0 \Rightarrow 160 F_C - F_A(160) = 0 \Rightarrow F_A = F_C = F$

② $\sum F_y = 0 \Rightarrow F_A + F_B + F_C = 160 \times 10^3$
 $2F + F_B = 160 \times 10^3$

③ Compatibility Equation: $\delta_A = \delta_C = \delta = \delta_B + 0.0003$

compatibility equation should be expressed in terms of forces (unknowns).

$$\frac{F_A(L_A)}{A_A E_A} = \frac{F_B L_B}{A_B E_B} + 0.0003$$

$$\frac{F(0.125)}{(400 \times 10^{-6})(70 \times 10^9)} = \frac{F_B(0.1247)}{(400 \times 10^{-6})(70 \times 10^9)} + 0.0003$$

$$125 F - 124.7 F_B = 8400$$

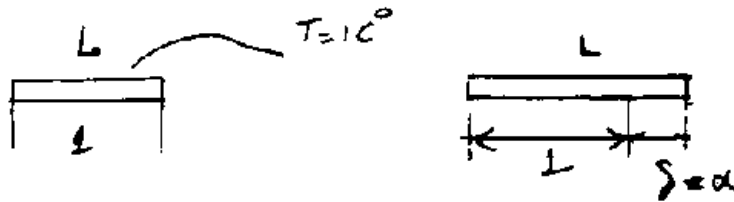
Solving ① & ② $\Rightarrow F = 75762 \text{ N}$
 $F_B = 8547 \text{ N}$

$$\sigma_A = \sigma = \sigma = \frac{F}{A} = \frac{75762}{400 \times 10^{-6}} = 189 \times 10^6 \text{ Pa} = 189 \text{ MPa}$$

$$\sigma_B = \frac{F_B}{A} = \frac{8547}{400 \times 10^{-6}} = 21.4 \times 10^6 \text{ Pa} = 21.4 \text{ MPa}$$

4.6 Thermal Stress

There are two types of stresses: one is called mechanical stress (due to physically applied load or reactions) and the other is thermal stress for restrained members (due to increase or decrease in temperature).

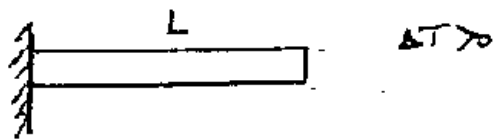


α = the change of unit length due to one degree of temp.
 = the strain due to one unit degree
 = coefficient of thermal expansion ($\frac{m}{m \cdot ^\circ C}$) or ($\frac{in}{in \cdot ^\circ F}$)

\therefore the total strain due to an increase in temp ΔT

$$\epsilon_T = \alpha \Delta T \quad (\text{thermal strain})$$

$$\Delta L = (\alpha \Delta T) L \quad (\text{change in length})$$



$$\epsilon_{\text{total}} = \epsilon_{\text{elastic}} + \epsilon_{\text{thermal}}$$

$$\epsilon_{\text{elastic}} = \epsilon_{\text{total}} - \epsilon_{\text{thermal}}$$

Hooke's Law: $\sigma = E (\epsilon_{\text{elastic}})$

$$\epsilon_{\text{total}} = \alpha \Delta T L$$

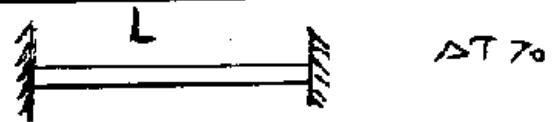
$$\epsilon_{\text{total}} = \epsilon_{\text{total}}$$

$$\epsilon_{\text{elastic}} = 0$$

$$\sigma = 0$$

$$\epsilon = \alpha \Delta T L$$

strain without stress



$$L \epsilon_{\text{total}} = L \epsilon_{\text{thermal}} + L \epsilon_{\text{elastic}}$$

$$\Delta L = \Delta L_{\text{thermal}} + \Delta L_{\text{elastic}}$$

$$0 = \alpha \Delta T L + \frac{NL}{EA}$$

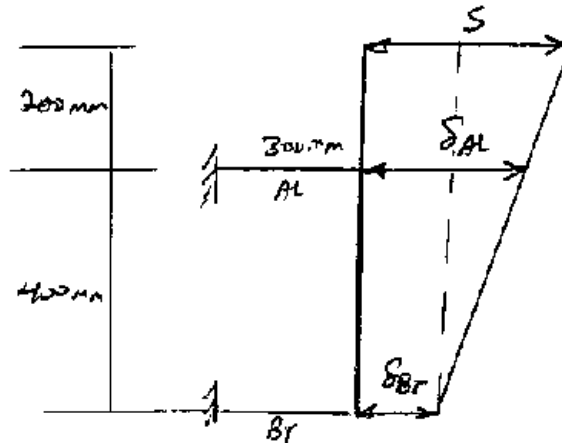
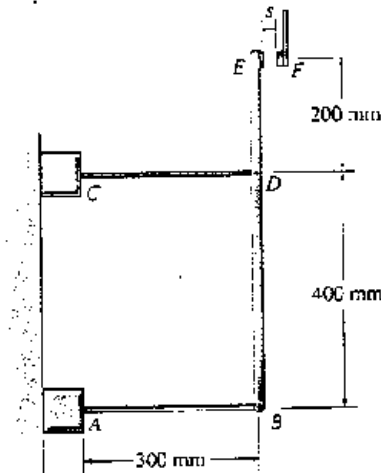
$$\Rightarrow N = -\alpha \Delta T EA$$

$$\sigma = \frac{N}{A} = -\alpha \Delta T E$$

$$\epsilon = \frac{\Delta L}{L} = 0$$

Stress without strain

4-74. The electrical switch closes when the linkage rods CD and AB heat up, causing the rigid arm BDE both to translate and rotate until contact is made at F . Originally, BDE is vertical, and the temperature is 20°C . If AB is made of bronze C86100 and CD is made of aluminum 6061-T6, determine the gap s required so that the switch will close when the temperature becomes 110°C .

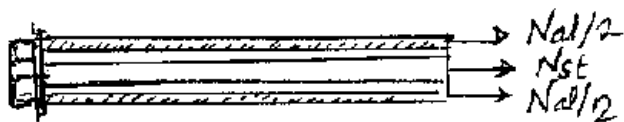


$$\begin{aligned}
 s &= \delta_{BR} + (\delta_{AL} - \delta_{BR})(1.5) = \frac{600}{400} \delta_{BR} \\
 &= -0.5 \delta_{BR} + 1.5 \delta_{AL} \\
 &= -0.5 \alpha_{BR} \Delta T L + 1.5 \alpha_{AL} \Delta T L \\
 &= (-0.5 \alpha_{BR} + 1.5 \alpha_{AL}) \Delta T L \\
 &= (-0.5 \times 17 \times 10^{-6} + 1.5 \times 24 \times 10^{-6}) (90) (300) \\
 &= 0.7425 \text{ mm}
 \end{aligned}$$

4-91. The steel bolt has a diameter of 7 mm and fits through an aluminum sleeve as shown. The sleeve has an inner diameter of 8 mm and an outer diameter of 10 mm. The nut at A is adjusted so that it just presses up against the sleeve. If the assembly is originally at a temperature of $T_1 = 20^\circ\text{C}$ and then is heated to a temperature of $T_2 = 100^\circ\text{C}$, determine the average normal stress in the bolt and the sleeve. $E_s = 200 \text{ GPa}$, $E_{al} = 70 \text{ GPa}$, $\alpha_s = 14(10^{-6})/^\circ\text{C}$, $\alpha_{al} = 23(10^{-6})/^\circ\text{C}$.



Prob. 4-91



$$A_{al} = 28.3 \text{ mm}^2$$

$$\Delta T = 80^\circ\text{C}$$

$$A_{st} = 38.5 \text{ mm}^2$$

$$E_{al} = 70 \times 10^3 \text{ MPa}$$

$$E_{st} = 200 \times 10^3 \text{ MPa}$$

$$\sum F_x = 0 \quad N_{st} + N_{al} = 0 \Rightarrow N_{st} = -N_{al} = N$$

As $\alpha_{al} > \alpha_{st}$, then aluminum will not elongate freely as it will without presence of steel. Therefore aluminum will be in compression and steel will be in tension.

compatibility:

$$\begin{aligned} \delta_{al} &= \delta_{st} \\ \frac{N_{al} l}{E_{al} A_{al}} + \alpha_{al} \Delta T l &= \frac{N_{st} l}{E_{st} A_{st}} + \alpha_{st} \Delta T l \end{aligned} \quad \begin{pmatrix} N_{al} = -N \\ N_{st} = N \end{pmatrix}$$

$$\frac{-N}{E_{al} A_{al}} - \frac{N}{E_{st} A_{st}} = \Delta T (\alpha_{st} - \alpha_{al}) \Rightarrow N = \frac{\Delta T (\alpha_{al} - \alpha_{st})}{\left(\frac{1}{E_{al} A_{al}} + \frac{1}{E_{st} A_{st}}\right)}$$

substituting $\Rightarrow N = 1134.4 \text{ N}$

$$\sigma_{st} = \frac{N_{st}}{A_{st}} = \frac{1134.4}{\frac{\pi (7)^2}{4}} = 29.5 \text{ MPa}$$

$$\sigma_{al} = \frac{N_{al}}{A_{al}} = \frac{-1134.4}{\frac{\pi (10-8)^2}{4}} = 40.1 \text{ MPa}$$